

COMPUTER IMPLEMENTATION OF OPSET— A SELF CALIBRATING WATERSHED MODEL

A Thesis Submitted
in partial Fulfilment of the Requirements
for the Degree of
MASTER OF TECHNOLOGY

By
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to the

DEPARTMENT OF CIVIL ENGINEERING
INDIAN INSTITUTE OF TECHNOLOGY KANPUR
DECEMBER, 1975

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ACKNOWLEDGEMENTS

I take this opportunity to express my deep sense of gratitude and profound regard to Dr. S. Ramaseshan and to Dr. V. Laxminarayana for their invaluable guidance and constant encouragement which were vital to the success of this effort.

Thanks are due to the Director, Indian Meteorological Department, Delhi, the Director, Agricultural Meteorology, Poona and the other concerned departments of the Government of India for supplying the necessary data to complete this work.

I want to express my thanks to the authorities and the staff of the Computer Centres at IIT Kanpur and at IIT Madras for their help at different stages of my computer work.

I am also thankful to my friends Messrs R. Katarya, N. Banerjee, P.P. Chattopadhyay, S.P. Rajagopalan, V.K. Sehgol, A. Debchoudhuri, Anil Nigam, Ravi Shankar, K.C. Dwivedi, K.K. Kedia, K.N. Paul, A.P. Majumdar and Miss K. Majumdar for their help from time to time.

Thanks are also due to Mr. J.C. Verma for drawing work and to Mr. J.K. Misra for neat and elegant typing.

SHYAMAL KUMAR RAY

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DICTIONARY OF VARIABLES

AEX90	Antecedent Evaporation Index, Decay Rate = 0.90
AREA	Area of Watershed
BFRC	Base Flow Recession Constant
BIVF	Basic Interflow Volume Factor
BMIR	Basic Maximum Infiltration Rate Within Watershed
BUZC	Basic Upper Zone Storage Capacity Factor
CHCAP	Channel Capacity - Indexed to Basic Outlet
CN	1 = A.M., 2 = P.M.
CONOPT	Control Option
CSRX	Channel Storage Routing Index
DATE	Current Day of the Month
DAY	Current Day of the Year
DIV	Diversion Into Basin, Mean Daily Flow
DPET	Dated Potential Evapotranspiration
DPY	Days per Year
DRHP	Dated Recorded Hourly Precipitation
DRSF	Dated Recorded Streamflow
DRSGP	Dated Recorded Storage Gauge Precipitation
ETLF	Evapotranspiration Loss Factor
FIMP	Fraction of the Watershed Being Impervious
FSRX	Flood Plain Storage Routing Index
FWTR	Fraction of the Watershed Being Water
GWETF	Ground Water Evapotranspiration Factor
HOUR	Current Hour of the Day
HRF	First Hour of Loop
HRL	Last Hour of Loop
IFRC	Interflow Recession Constant
ISGRD	Current Storage Gauge Rainfall Day
IWBG	Index Number of Weather Bureau Precipitation Gauge

KRD	Counter for Reading Data Arrays
KWM	Kentucky Watershed Model
LZC	Lower Zone Storage Capacity
LZS	Current Lower Zone Storage
MEDCY	Month End Dates - Calendar Year
MNRC	Minimum Number of Rough Cycles
MONTH	Current Month of the Year
NCTRI	Number of Current Time Routing Increments
NPTR	Number of First Trip to Be Run For a Given Station Year
NLTR	Number of Last Trip To Be Run For a Given Station Year
NSGRD	Number of Storage Gauge Rainfall Days
OFMN	Overland Flow Manning's N
OFMNIS	Overland Flow Manning's N, Impervious Surfaces
OFL	Overland Flow Surface Length
OFSS	Overland Flow Surface Slope
SIAC	Seasonal Infiltration Adjustment Constant
SIK	Current Storage Routing Index
SQDM	Sum of the Squares of the Monthly Flow Deviations
SUBWF	Subsurface Water Flow Out of the Basin
SUZC	Seasonal Upper Zone Storage Capacity Factor
SWM	Stanford Watershed Model
'RIP	Counter Specifying Program Portions
UZC	Upper Zone Storage Capacity
INTMR	Vegetative Interception - Maximum Rate
EAR	Last Two Digits of Current Year

SYNOPSIS

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COMPUTER IMPLEMENTATION OF OPSET - A SELF CALIBRATING WATERSHED MODEL

The present study is basically related to the implementation of OPSET, a self calibrating watershed model program, to the computer system IBM 7044 and IBM 370/155 and the application of this to an Indian watershed to find out the optimum set of some parameter values which influence the runoff in a watershed greatly. Test data are available to implement the program. For the Indian watershed the measurable watershed parameters and other climatological data are collected from different places.

Physical description of the watershed under study is given and the definitions and magnitudes of the measurable watershed parameter values are also presented here. While optimizing the parameter values the main objective was to synthesize the streamflow and hydrograph peaks as closely as possible with those of recorded values respectively. Synthesized and recorded values of flows are tabulated for comparison.

Changes are made inside the program where necessary and are given in this study. Ultimately the optimized parameter values obtained from OPSET program can be used in the Kentucky Watershed Model to use in ungauged watersheds.

CHAPTER I

INTRODUCTION

1.1 Hydrologic Cycle and Processes:

Hydrology is a branch of physical geography. It deals with the origin and distribution of the waters of the earth.

The study of hydrology starts with the concept of hydrologic cycle. Water evaporated from the oceans and the free water surface of the land mass is transported by moving air masses. Under proper conditions the vapour is condensed to form clouds, which, in turn, may result in precipitation. The precipitation which falls upon land is dispersed in several ways. Part of the water is returned to the atmosphere by the processes of evaporation and transpiration. Of the remaining part, a portion passes over and through the surface soil to the stream channels and other portion penetrates deeper inside the earth to become part of the earth's ground water supply. Under the influence of gravity these surface waters and ground waters always move towards the lower elevation but a substantial quantity of these return to the atmosphere through the processes of evaporation and transpiration.

A review of the hydrologic cycle, particularly of those processes which are important in governing runoff rates, provides the best background for understanding the model in

this study namely OPSET, the basic logics of which are dependent on the Stanford Watershed Model. The processes are described briefly in the following paragraphs.

Moisture held on vegetative surfaces is known as Interception Storage. This is dependent on type and density of vegetative cover. This reduces the runoff but its effect is less in large storms compared to small storms.

Precipitation not held by vegetative cover but held in hollows and behind ridges on soil surface is known as Depression Storage. This is also more effective in reducing runoff during small storms than during larger storms. Depression storage capacity is generally larger than interception storage capacity and is governed primarily by the roughness of the soil surface, human activity and watershed topography. Steep slopes reduce the depression storage capacity. In arid climate the less frequent runoff does not wash out the ridges and depression storage is greater.

A portion of the water in contact with the soil surface infiltrates into the soil which may return to the stream channel as Interflow or Baseflow or may be used by vegetation as evapotranspiration. Infiltration into the ground depends generally on the soil permeability, surface characteristics, presence or absence of vegetation, etc. But freezing of the upper zone, rising of the underground water table and low permeability at moderate depths are some factors for reduced infiltration rate.

High evapotranspiration rates will deplete the soil moisture between storms and consequently increase the infiltration rate. Evaporation from soil surfaces and exposed water surfaces depends on the degree of exposure, air temperature, water temperature, wind velocity, atmospheric pressure and percent solids in the water. Moisture loss through transpiration depends upon the degree of vegetative cover and other climatological factors. Urban development greatly increases the relative imperviousness of a watershed.

Water which does not percolate to the water table (to later appear as baseflow) may proceed toward the stream in two ways: (1) it may travel over the land surface (overland flow); or 2) it may travel partially through and partially above ground (interflow). The time lag of overland flow is governed by the slope, distance to the nearest collector stream and roughness of the soil surface. Overland flow is very rapid in impervious surfaces.

If the water table is higher or if there is an impervious layer at a shallow depth, the interflow increases. Rapid flow through the soil layers is seen in most permeable soils. Quantity of interflow is dependent on the local infiltration capacity.

Overland flow, interflow, baseflow enter the channel system at different, scattered points in a watershed. The

channel inflows combine as they flow downstream and ultimately reach the gauging station in the stream. This channel flow rate depends on the slope, size, shape, and hydraulic roughness.

A schematic diagram of the hydrologic cycle is shown in Fig. 1.

1.2 Mathematical Modeling and Analysis:

Full synthesis of the hydrologic cycle is practically impossible due to vast complexity of the systems involved in the study, inadequacy of the knowledge now available and the knowledge likely to exist in the foreseeable future. The other way is to solve the practical technological problems by establishing workable relationships between measurable parameters in the hydrologic cycle.

'Systems investigation' is needed in hydrology to establish quantitative relationships between precipitation and run-off, which can be used for 'reconstruction or prediction (ROP) of flood sequences and watershed yields. System investigation methods are (a) parametric and (b) stochastic. In the present work parametric study is done.

Parametric hydrology is the development of relationships among physical parameters involved in hydrologic events and the use of these relationships to generate, or synthesize, non-recorded hydrologic sequences [1]. Among all the studies

UPPER ZONE STORAGE = INTERCEPTION $P - us$ DEPRESSION STORAGE

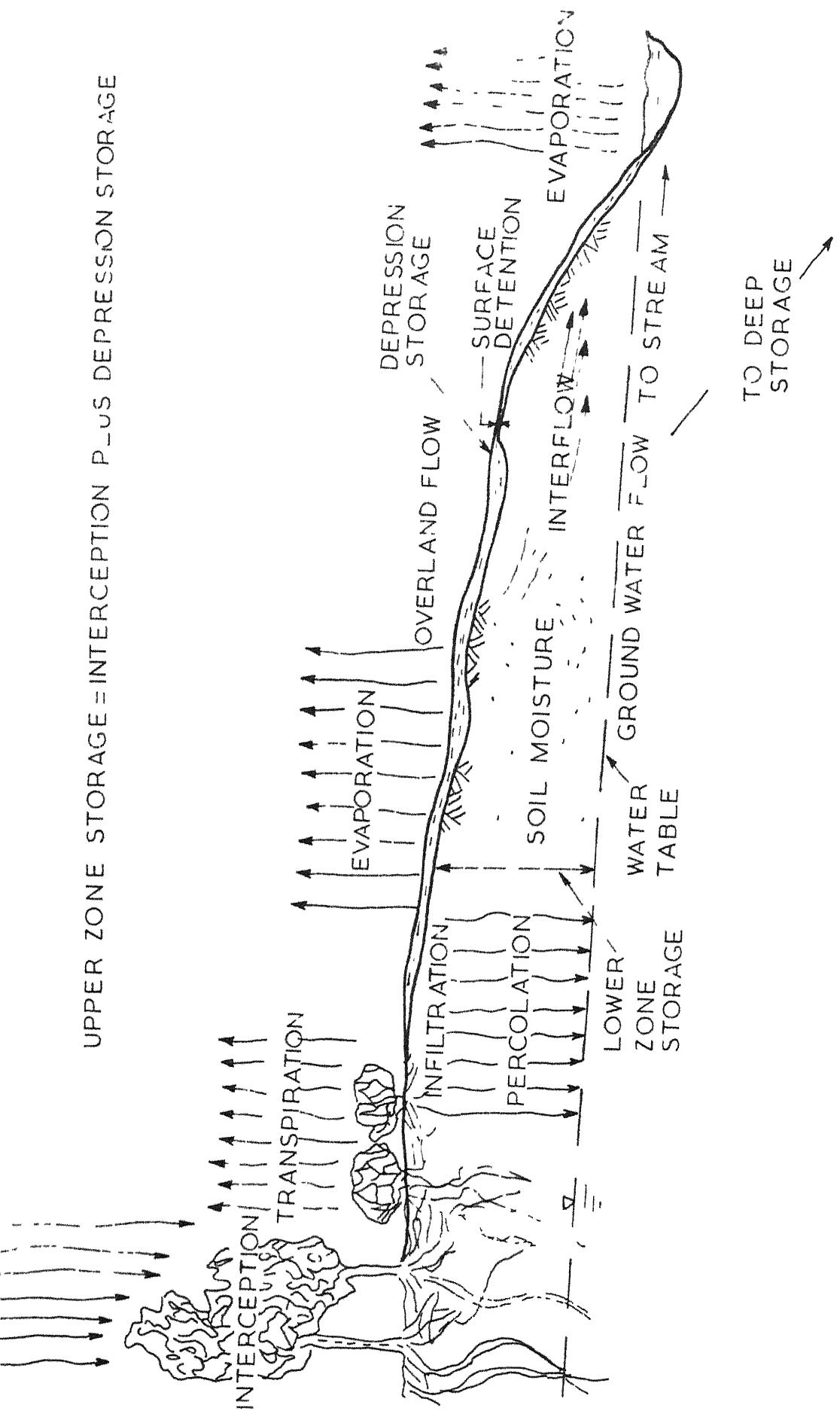


FIG.1 SCHEMATIC OF HYDROLOGIC CYCLE

made in parametric hydrology only the general non-linear analysis is independent of detailed knowledge of physical hydrology where emphasis is centred on topics of study. In the method of correlation analysis various combinations of variables are tested to explore the significance of their effects in the hydrologic system. The combination, among those investigated, that yields a relationship most closely approximating the recorded output function in terms of the recorded input function and other arbitrary parameters is adopted as the best prediction equation.

In system analysis, the synthetic models in hydrology are dependent on the continuity equation of the form,
 $I = Q + \Delta S$, where I = total inflow, Q = total outflow and ΔS = change in internal storages for any given time interval. Any system like this, defined by the continuity of matter, is known as 'closed' system.

Through the general non-linear analysis, which started a few years ago, one can find out the relationships between partial inputs and partial outputs to be established independently, subject to some conditions of mathematical continuity and boundedness. It is not necessary to satisfy physical continuity conditions among the total input, total output and inner storage. When input and/or output represents only partial form of the total input or total output of the system, the system is called 'open'. Considering a hydrologic unit as an

open system one has obvious advantage in that it bypasses the need to evaluate component inputs and outputs.

Hydrologic system cannot be treated as analytic system because current mathematical knowledge is insufficient to evaluate the nonlinear kernels of the functions of series when multiple inversion is not possible due to complex input.

Physical simulation, for a hydrologic system, is not possible due to limited data. Ultimately digital simulation helps in this regard.

1.3 Simulation.

To simulate means to duplicate the essence of the system or activity without actually attaining reality itself. It is used to circumvent the difficulties of duplication of environment, of mathematical formulation, of lack of analytical solution techniques, or of experimental impossibilities.[2].

In hydrology simulation is used to correlate the observed hydrological behaviours to the factors which influence these and to predict the behaviour of watersheds where observed data are unavailable or incomplete. For this, large quantities of physical data on rainfall, evapotranspiration, streamflow, and other related variables are collected. Qualitative knowledge about the hydrological cycle is reasonably complete and interaction between major components such as rainfall intensity and infiltration rates can be logically explained. Still the

extension of qualitative knowledge to quantitative procedures is challenging. The land phase of the hydrological cycle contains complex interactions that are difficult to document from observed data, and the volume of data that must be analysed is formidable. Streamflow simulation attempts to develop algorithms of hydrological processes for quantitative calculations. In the present study hydrological reactions are assumed deterministic.

Since we are dealing with a physical system, a digitally simulated model deals with partial differential equations for rainfall, evapotranspiration, streamflow etc. This is too complex. Further, detailed and accurate data for soil moisture, overland flow etc. are not available in our country. Therefore, hydrologic basin models with lumped elements have been used with different levels of details.

1.3.1 Streamflow simulation models:

There are many models for streamflow simulation, few of which are described here. Three types of models are generally used these days. These are (a) Mathematical Models, (b) Lumped Models and (c) Conceptual Models.

In the study of hydrology with mathematical modeling, run-off process can be analysed. Hydro meteorological characteristics which define the behaviour of catchments and the construction of mathematical models which approximately

reflect the process of the runoff are complementary. In modeling runoff some difficulties are encountered such as lack of basic data combined with considerable noise level and uncertainty of some links in the hydrologic process namely the characteristics of water movement in the drainage basin, its depth and the circumstances of its emergence on to the surface, followed by routing through the channel. To overcome all these, some hypotheses in models are used to match model with the natural data. The simplest of this kind is a model of snow-melt runoff on the surface of a mountain.

In Retention Model, a catchment is regarded as an interconnected system of catchment units and channel units. Each catchment unit consists of a retention storage and a runoff storage. The retention storage does not meet the stream flow and is composed of capillary soil moisture, precipitation intercepted by vegetation, and some parts of the depression, ground water and channel storage. The runoff storage consists of the water that is likely to become streamflow, including overland flow, 'free' water moving through soil or litter, ground water and storage in minor channels. Informations of leakage, channel transmission losses are taken into account, if available. This model was originally suggested for analysing the effects of vegetation and soils on runoff volumes but it seems just as suitable for short term forecasting of floods due to rainfall [3].

The objective of a Conceptual Catchment Model is to simulate as accurately as possible the streamflow hydrograph under natural conditions and also to evaluate the modifications to natural flows resulting from man-made physical changes or management practices. Ideally, a conceptual catchment model should satisfy the following four conditions for maximum usefulness in river forecasting:

- (1) It should represent the significant hydrological processes in a rational manner;
- (2) It should contain the minimum number of parameters required for adequate representation of the processes;
- (3) The values of the parameters should be measurable or be significantly correlated to easily measurable characteristics and
- (4) The model should contain procedures for updating as new information becomes available, in a rational if not optimal manner.

There is still much to be learnt about the various phases of the hydrologic cycle and research is being carried out to increase the knowledge of them.

1.3.2 Conceptual models:

The basic philosophy of some of the well known conceptual models are presented below.

1. Stanford Watershed Model (SWM):

This is the best known of the current conceptual models. A brief description only is given here and the fuller description is given in Chapter 2. The SWM simulates continuously the movement of water as it moves over, into and through the soil. It uses as input rainfall, snowmelt or both. Interception and storage losses must be satisfied initially. A portion of the runoff comes over the impervious surface and joins the streamflow and the rest passes over the pervious surface and is subject to the soil moisture conditions. Infiltration capacities vary from watershed to watershed. Detention storage, overland flow and interflow are due to the accumulation of water on the upper zone. Water moves from upper zone to lower zone and ultimately joins the ground water storage. Water stored in the soil moisture zone is called lower zone storage. Lake evaporation from class A pan records is assumed to be potential evapotranspiration. Evapotranspiration is assumed to take place from interception storage, upper zone, lower zone and ground water storage but at different rates. Ground water runoff, interflow and surface runoff are routed separately and combined to produce outflow hydrograph. Hydrological processes are represented mathematically and the parameters involved in these mathematical quantities are obtained by the digital computer.

2. The Utah State Simulation Model:

This simulation model has been developed by the Utah State University. The computer needed for its solution is an analog computer. Its objectives are (i) the development of improved relations for describing the various hydrological processes and inter-connecting link between these processes, and (ii) the development of an analog computer having a high degree of flexibility and capability for the solution of hydrological and related problems.

The hydrologic balance of this model is given by,

$$P = I + \Delta M_s + SRO + \Delta G_s + GWO + ET \pm \Delta S_s$$

where,

P = Precipitation,

I = Interception loss,

ΔM_s = Change in soil moisture storage,

SRO = Surface run-off,

ΔG_s = Change in ground water storage,

GWO = Ground water run-off,

ET = Evapotranspiration, and

ΔS_s = Change in surface storage.

Interception loss is a function of type and density of cover and precipitation. SRO is the excess of precipitation over the infiltration rate. Soil moisture storage is a function of infiltration, evapotranspiration, interflow and percolation

to ground water storage. The relation of actual to potential evapotranspiration rate is assumed to be a function of soil moisture deficiency. Percolation of ground water is dependent on soil moisture storage. Ground water runoff is a function of ground water storage. The increments of surface runoff, inter flow and ground water are routed separately through different amount of storage.

3. The Agricultural Research Service Model:

This model, responsible for agricultural research in the U.S., is based on infiltration theory approach. In this model first the depression storage on the surface must be satisfied. An assumption is made that some impeding stratum of the soil acts as a control on saturated flow. Therefore, the infiltration capacity (f) of a surface soil approaches the low constant seepage rate (f_c) of the impeding stratum as the overlying storage (S) is exhausted by infiltration volumes (F). The equation is as follows:

$$f = \alpha (S - F)^{1.4} + f_c$$

where, α = percentage of area occupied by plant crowns, because plant roots connect large pores and provide continuous channel.

For this model, soil data are needed which permit determinations of soil porosities and occurrence of impeding strata.

Soils are grouped in accordance with their constant rate of infiltration after prolonged wetting. This constant rate is assumed as the seepage rate through the impeding strata. Porosities are classified as (i) pores (h), free water drainable by gravity, and (ii) pores, available water capacity (AWC), drainable by evapotranspiration. Recovery of available storage, by this means, is at the rate of seepage (f_c) to the extent of freely drainable porosity. Further recovery of available storage capacity between the days of rain is at the rate of evapotranspiration (ET) to the extent of plant available water (AWC). Different values of a 's, plant root coefficients, are prepared relative to various land use and treatments. Potential ET is estimated from pan-evaporation with a seasonal consumptive use coefficient applied to obtain the actual evapotranspiration. Water in excess of infiltration and surface depression storage is assumed available for run-off and is routed by successive routing techniques to reproduce the outflow hydrograph.

4. The Columbia River Basin Model:

The input to this model is weighted rainfall plus computed snowmelt on a daily basis. This daily input is distributed into specific time period (3, 6 or 12 hours). The total runoff is calculated by multiplying rainfall, snowmelt or both by a percent coefficient which is a function of soil

moisture index. The soil moisture index at the end of a period is the soil moisture index at the beginning plus precipitation minus total run-off minus evapotranspiration for the same period. Evapotranspiration on rainy day is less and is obtained by multiplying the daily value by a coefficient. The percent of the total runoff contribution to base flow is computed as a function of a base flow infiltration index. The remaining runoff is separated into surface and subsurface components as a function of the total input ratio. Components of surface, subsurface and baseflow runoff are routed separately, using a successive reservoir routing technique. It is also possible in this model to introduce rule curves for operation of reservoirs and/or to specify release schedules for all or selected reservoirs.

Studying all the above models it is observed that the Stanford Watershed Model is the best suited in this study, considering the availability of data. The Kentucky version of the Stanford Watershed Model known as Kentucky Watershed Model (KWM) was available with us for analysis.

KWM has a number of parameters. Estimation of these parameters is very time consuming. 'A self calibrating model' known as OPSUT is available for finding the best fit of values for the parameters so that simulated flow matches the recorded flow. This model uses hourly rainfall data, daily streamflow

data and yearly, daily or 10-daily average evaporation data besides several other types of data.

In India, there are nearly 400 self recording rain gauge stations. The data from these stations are yet to be used in detailed hydrologic modeling of basins. It seems possible to use KWM in order to model basins with hourly rainfall data. Hence, it is proposed to implement the OPSET program developed by the University of Kentucky and available with us and use it, if possible, to model a basin for which data are available.

1.4 Statement of the Problem:

The objectives of the study are

- (1) to implement the OPSET programme for estimation of parameters of the KWM using the test data given in the original report [4], and
- (2) to estimate the parameters for a basin in India with the requisite data.

1.5 Scope of the Study:

- (1) The implementation of OPSET on IBM 7044 and IBM 370/155 computer systems is done using the set of available test data.
- (2) Estimation of parameter values for a basin in India is done on the basis of available data.

Within the limitations of time, computer facilities, money and data availability, parameters are determined only for 2 years for the Indian basin.

CHAPTER II

1

DETAILS OF OPSET PROGRAM

2.1 Kentucky Watershed Model:

2.1.1 General Description:

The Kentucky Watershed Model (KWM) is the translated, revised and expanded version of the Stanford Watershed Model. Originally, the Stanford Watershed Model was written in a digital computer language SUBALGOL. James [5,6,7] translated it into FORTRAN IV and modified it. This version is known as KWM. As the basic logic of SWM and KWM are same, the description of Stanford Watershed Model is given here.

SWM is a systematic mathematical representation of the hydrologic processes in the hydrologic cycle. Mathematical formulations in the model are empirical for some of the processes.

In this model the major input data are evapotranspiration and rainfall. If snow is a significant part of precipitation, then snowfall data should also be given as input data. In addition, several other input parameters and control arrays must be selected based on Watershed behaviour, characteristics and data availability. The details of input parameters are given by James [5].

Output consists of synthetic streamflow, overland flow, interflow, baseflow, stream evapotranspiration (net and potential) and ground water storage. These hydrologic quantities can be obtained daily, monthly or annually as appropriate for use by the researcher.

2.1.2 Flow Chart of KWM:

A schematic flow chart of KWM is given in Fig. 2. In the above mentioned figure the boxes represent classifications of moisture storage. Arrows are the processes through which moisture moves from one type of storage to another type of storage. The vertical straight line from precipitation to subsurface flow with arrow downwards represents the flow of water through the porous media due to gravity. When moisture cannot move downward due to some restrictions, such as low permeability, moisture starts accumulating in the higher boxes. Water moves laterally and joins the stream when storage capacities of these boxes are exceeded. Ultimately, it goes out of the watershed. Vertical straight line with arrow upward indicates the withdrawal of water from all levels of storage due to radiation energy, and moisture needs of vegetation.

The path taken by incoming moisture is determined by the antecedent moisture storage, the magnitude of the assigned parameter values, the entry rate and the season of the year.

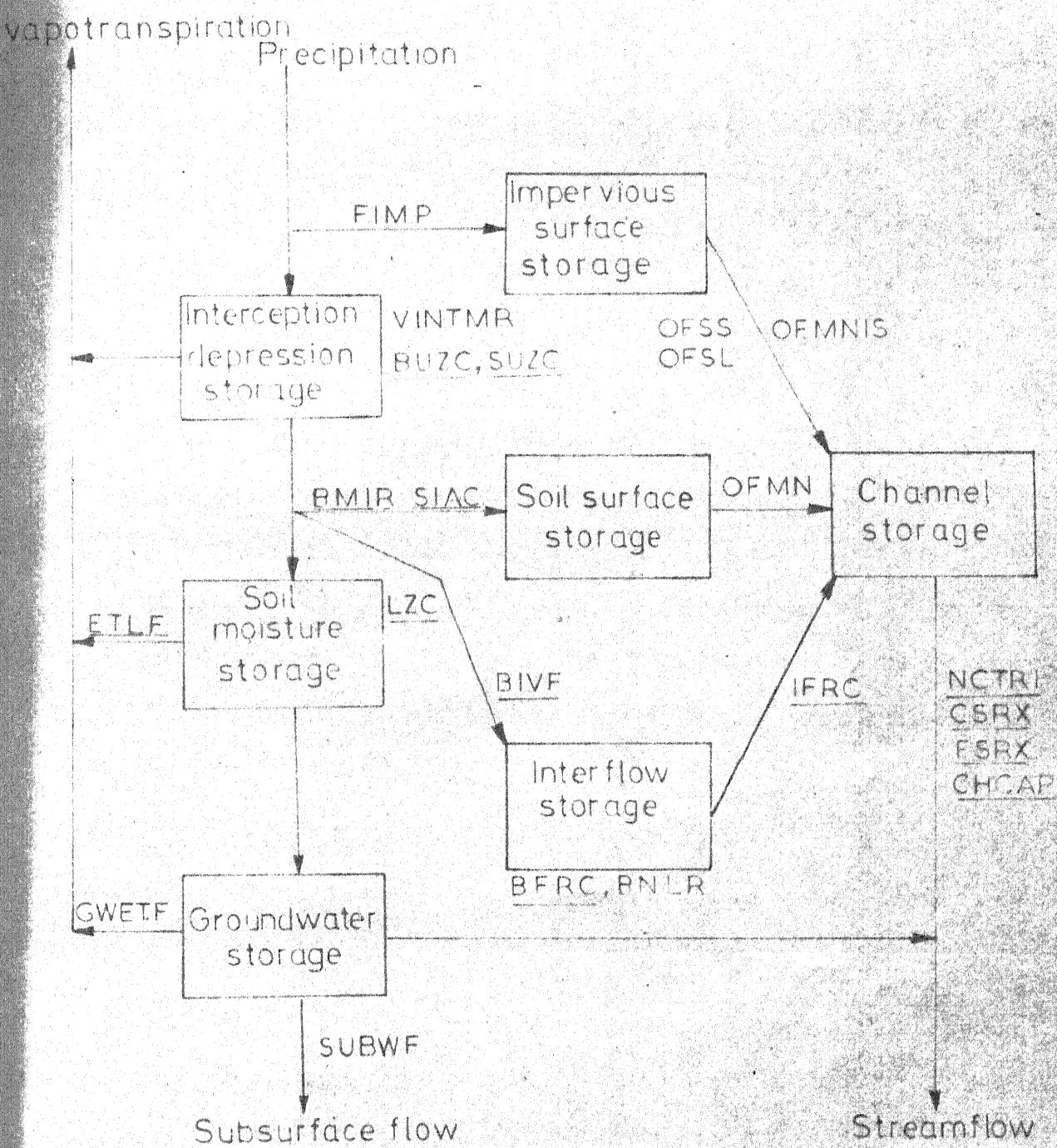


FIG.2 SCHEMATIC FLOW CHART OF KWM

The quantities appearing along the flow lines are the parameters which control the flow of moisture along those paths.

2.1.3 Input Data to KWM:

The input data required by SWM or Fortran KWM are divided into 6 groups as follows:

1. Data to specify the desired program options and required specific output.
2. Data to initialize the watershed soil moisture storage condition.
3. Data to establish climatological events.
4. Time-area histogram for watershed under study.
5. Data to assign values to the watershed parameters,
6. Recorded streamflow to compare with the synthesized flow.

2.2 OPSET:

2.2.1 Definition of OPSET and Purpose of KWM:

OPSET is a self calibrating model. It is called OPSET because of its objective to estimate the Optimum SET of parameter values. Input parameters are those which can be directly measured by the user. Other parameters are repeatedly adjusted by OPSET to match the synthetic hydrograph with observed hydrograph until no further adjustment is possible. Optimum set of parameters are obtained from OPSET for different water years.

for a watershed and then averaged. They can be used in KWM. KWM, in order to synthesize streamflows, uses these parameter values with much larger number of control options. A schematic diagram of parametric optimization procedure followed by OPSET program is shown in Fig. 3.

2.2.2 Parameters Estimated by OPSET:

There are altogether 13 parameters to be determined by OPSET which are classified as follows. These parameters are underlined in Fig. 2.

Recession Constants:

1. IFRC - Interflow Recession Constant
2. BFR - Baseflow Recession Constant

Land Phase Parameters:

(a) Runoff Volume Parameters:

LZC - Lower Zone Storage Capacity,
BMIR - Basic Maximum Infiltration Rate Within Watershed,
SUZC - Seasonal Upper Zone Storage Capacity Factor,
ETLF - Evapotranspiration Loss Factor,
BUZC - Basic Upper Zone Storage Capacity Factor,
SIAC - Seasonal Infiltration Adjustment Factor.

(b) Surface Volume Parameters:

BIVF - Basic Interflow Volume Factor

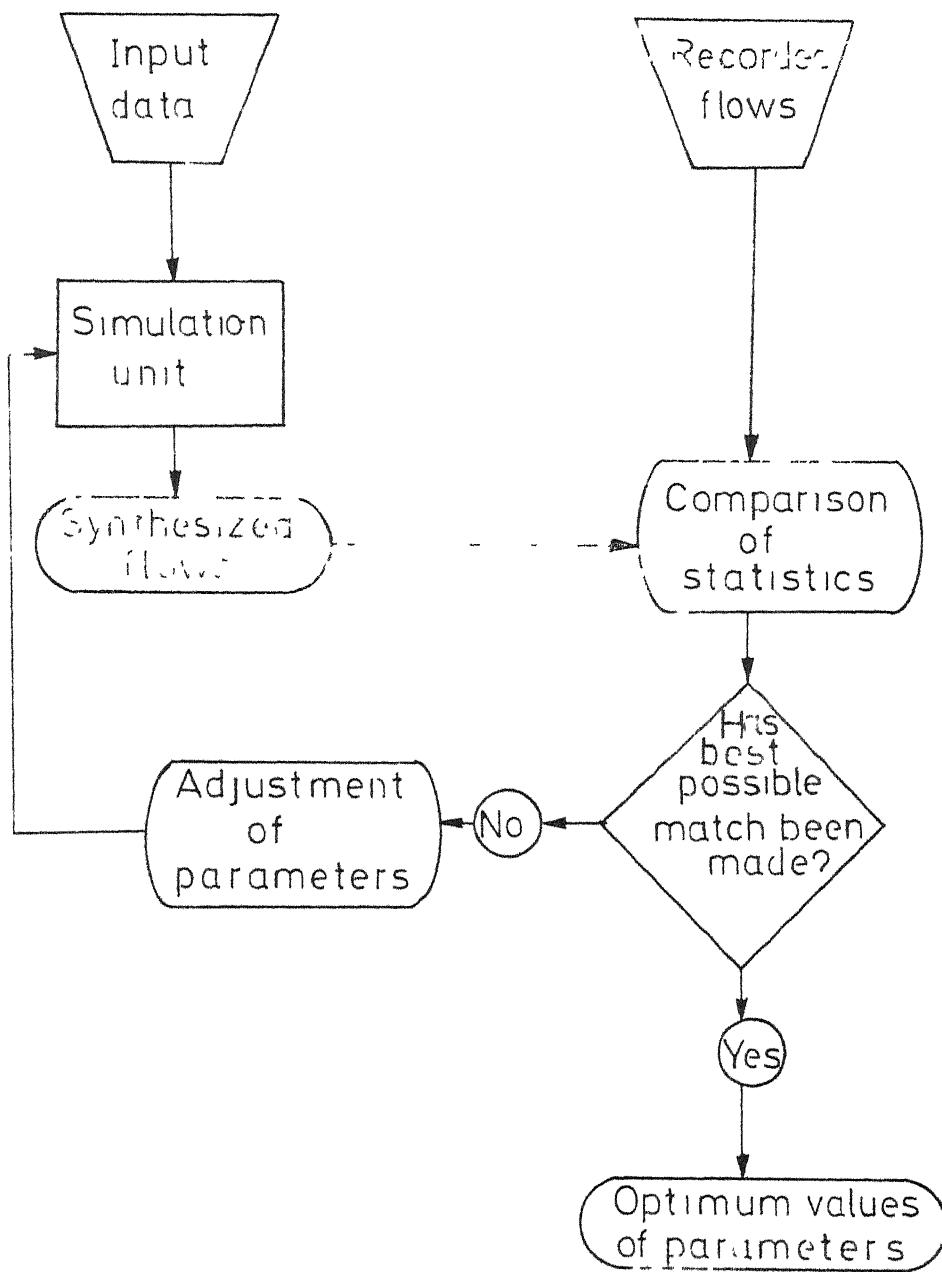


FIG.3 SCHEMATIC DIAGRAM OF PARAMETER OPTIMIZATION PROCEDURE

Channel Routing Parameters:

1. NCTRI - Number of Current Time Routing Increments,
2. CSRX - Channel Storage Routing Index,
3. FSRX - Flood Plain Storage Routing Index,
4. CHCAP - Channel Capacity - Indexed to Basin Outlet.

2.2.3 Physical Significance of Parameters:

Over a real watershed, the hydrologic processes are continually going on at rates varying with time and location. The movement of moisture at any point is in response to acting forces (gravity) counteracted by the resistance to moisture movement along a given flow path. The resultant moisture moves in low resistance zone. Physical factors governing the saturated and unsaturated flows include the size and shape of the particles, the porosity, orientation, and the moisture content of the area, viscosity and surface tension of water and information on how these factors vary over time and space. In this model lumped estimation process has been used. Option is there in SWM to divide the whole area into a number of segments. The estimate should also be lumped over a finite interval of time (15 mins. in SWM). They also can be lumped to a group of related hydrologic processes such as interception storage is added to depression storage in the SWM. Baseflow recession constants represent the cumulative effect of moisture movement along a large number of routes. Lumped

estimates imply that watershed parameters somehow aggregate the effects over space and time. Best values are those which give the synthesized flow close to the recorded flow. But this depends on the data measurement error, size of the area and length of time represented etc. One point is very important that the parameters which are best in 15 minutes time increments will not be the best in 60 minutes time increments [8].

2.2.4 Parameter Estimation Criteria:

There are two criteria to choose the parameters which are to be estimated by the OPSET program. First, the parameters should be difficult to measure directly. Parameters like drainage area should not be estimated by matching synthetic flow with the recorded flow as it is easy to measure this to a satisfactory degree of accuracy. Second, the simulated flow sequences should be sensitive to the parameters. Considering the above two criteria the thirteen parameters mentioned before were chosen.

2.3 Parameter Estimation:

The procedure followed by OPSET program to estimate the parameters is outlined below.

2.3.1 Recession Constants (BFRC and IFRC):

Traditional graphical techniques to find out BFRC (base low recession constant) and IFRC (inter flow recession constant)

are time consuming. The recession sequence is selected by the subroutine RECESS. Maximum length of each sequence is 50 days and for each station year upto 20 flow sequences are selected. Minimum number of days needed to estimate a single recession constant is 2 and for 2 constants 4 days are needed. Longer sequences give better results. As recessions from very small flow do not give good results a criterion is used that the second day flow should be either greater than 10 cfs or greater than $0.4 \times \text{AREA}$ (where AREA is the watershed area in sq. miles). Second day is the day after peak or the first day whose flow is actually used to find recession constants. The recession sequence is terminated by a flow rise exceeding $0.1 \times \text{SQRT (AREA)}$ cfs. After finding out the recession sequences through the subroutine RECESS the subroutine SET2RC and SET1RC are used to find the values of IFRC and BFRC.

Subroutine SET2RC is called first to estimate the two recession constants IFRC and BFRC. If the sequence starts with a relatively low flow and flat recession then it is assumed that only base flow is present and subroutine SET1RC is used. Here BFRC is found. If the value of BFRC does not come up between 0.6 and 1.2 the entire sequence is discarded.

2.3.2 Runoff Volume Parameters (LZC, BMIR, SUZC, ETLF, BUZC, SIAC):

In order of decreasing sensitivity these parameters are LZC, BMIR, SUZC, ETLF, BUZC and SIAC. The subroutine SETFVP

adjusts the values of five flow volume parameters LZC, SUZC, ETLF, BUZC and SIAC during the process of estimating the best set of values for the six flow volume parameters. In order to see how and to what degree each of these parameters affects runoff, a sensitivity study was made with the data for Elkhorn Creek, Frankfort, Kentucky, USA, for the year 1964. First of all a best set of parameter values were selected by trial and error. Then for each of six parameters 2 computer runs were made by varying that parameter keeping others constants. Each parameter had effect on simulated flow and differed with the changes of type of flow and changes in time of the year. BMIR is adjusted through the subroutine SETBMI and other five through SETFVP. These two subroutines function as unit and adjusts the six volume parameters simultaneously.

First of all, trial values are taken for these parameters and a year of streamflow is simulated. Then each parameter is adjusted by its adjustment rule. The new set of six values is used to simulate another year of streamflows, and this process continues until the simulated flows have smaller SSQM as computed by subroutine SETFDI. Recorded and simulated flows cannot match perfectly due to data and modeling difficulties.

Runoff volume parameters are defined as follows:

1. BUZC:

It is an index for estimating the capacity of the soil surface (upper zone) to store water in interception and depression storage. Within the underlying layer more will be the amount of water stored in this layer.

2. SUZC:

It is an index for estimating soil surface moisture storage capacity. Seasonal storage capacity changes due to summer vegetation and cultivation is taken care of by SUZC. BUZC and SUZC are used to compute the upper soil zone nominal storage capacity (UZC) by the following equation.

$$UZC = SUZC (AEX90) + BUZC (e^{-2.7} \times \frac{LZS}{LZC})$$

where,

AEX90 = Antecedent evaporation index ($K = 0.90$),

$\frac{LZS}{LZC}$ = An index of the moisture content of the underlying soil,

e = Natural logarithm base.

3. LZC:

It is the soil-moisture storage capacity index which approximately equals the volume capacity of the soil to hold water. Decreasing LZC will be reducing the ability of soil to hold water and thus increasing the synthesized flow and the reverse when the value of LZC increases. Delayed infiltration

or percolation happens from the upper zone to the ground water and lower zone storages when UZS/UZC exceeds the value of LZS/LZC (Ref. Figs. 4 and 5).

4. ETLF:

ETLF is an index used to estimate the maximum rate of evapotranspiration which could occur within the watershed under current conditions of soil moisture content. This maximum rate is then used to estimate current actual evapotranspiration in the manner depicted in Fig. 6. More trees in a watershed means higher value of ETLF and less simulated flows (Ref. Fig. 6).

5. SIAC:

It is an evaporation-infiltration factor relating infiltration rates to evaporation rates to account for more rapid infiltration during warmer period. Higher the value of SIAC less is the runoff.

6. BMIR:

It is the basic infiltration index used to control the rate of infiltration. Infiltration rates vary from point to point in a watershed and most runoff will be from points with smaller rates. A cumulative frequency distribution is used in SWM, varying linearly from zero to a maximum value (Ref. Fig. 7).

The equations relating maximum capacity to infiltration rates, applicable at any particular point in time (CMIR), are given by,

$$\begin{aligned}
 \text{CMIR} &= \frac{\text{Constant} \times \text{SIAM} \times \text{BMIR}}{\text{Function (LZS/LZC)}} \\
 \text{SIAM} &= \text{Function (SIAC)} \\
 \text{CIVM} &= \text{BIVF} \times \text{Function (LZS/LZC)}
 \end{aligned}$$

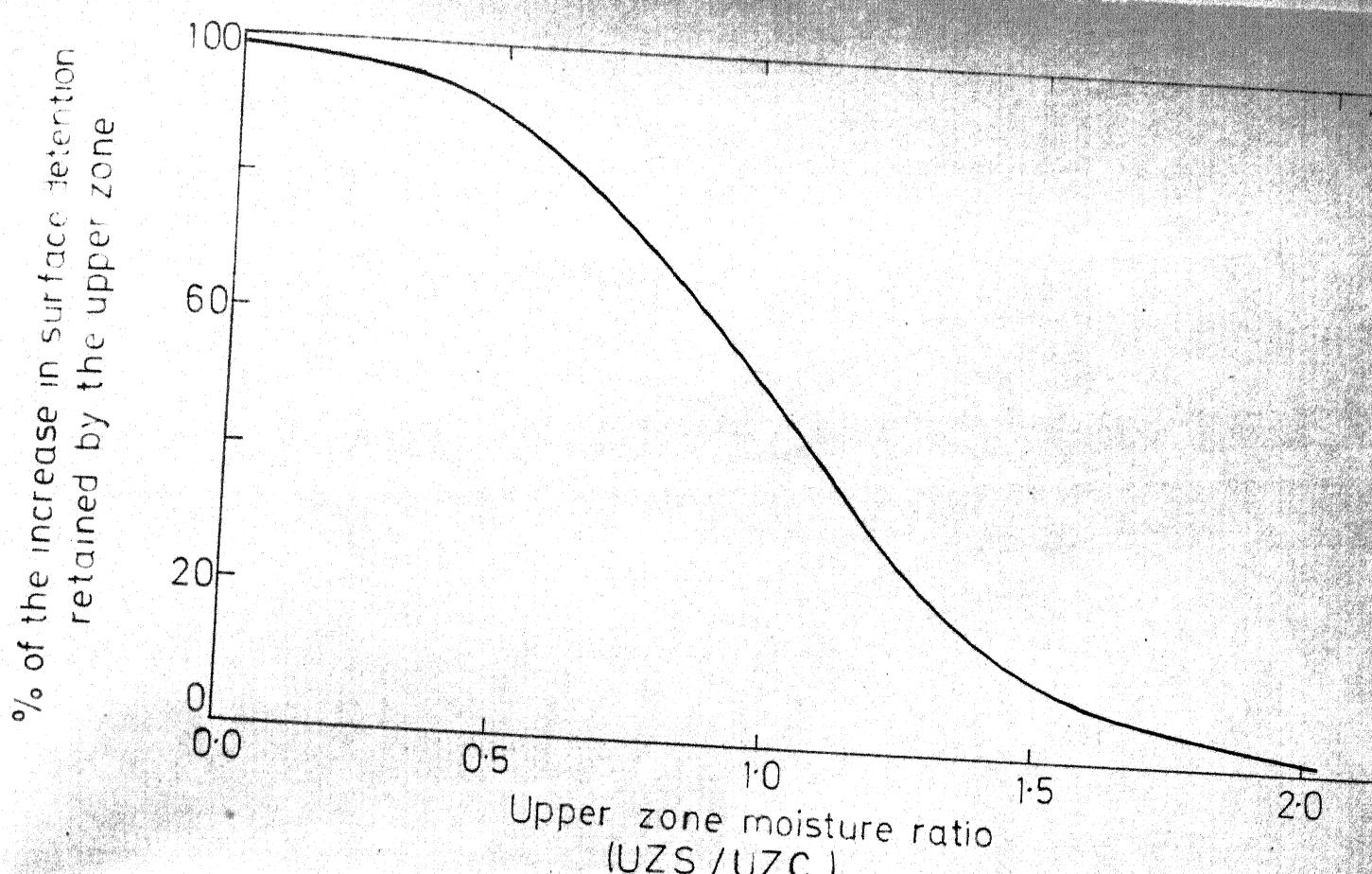


FIG.4 MODEL FOR ESTIMATING THE UPPER ZONE STORAGE COMPONENT OF SURFACE DETENTION

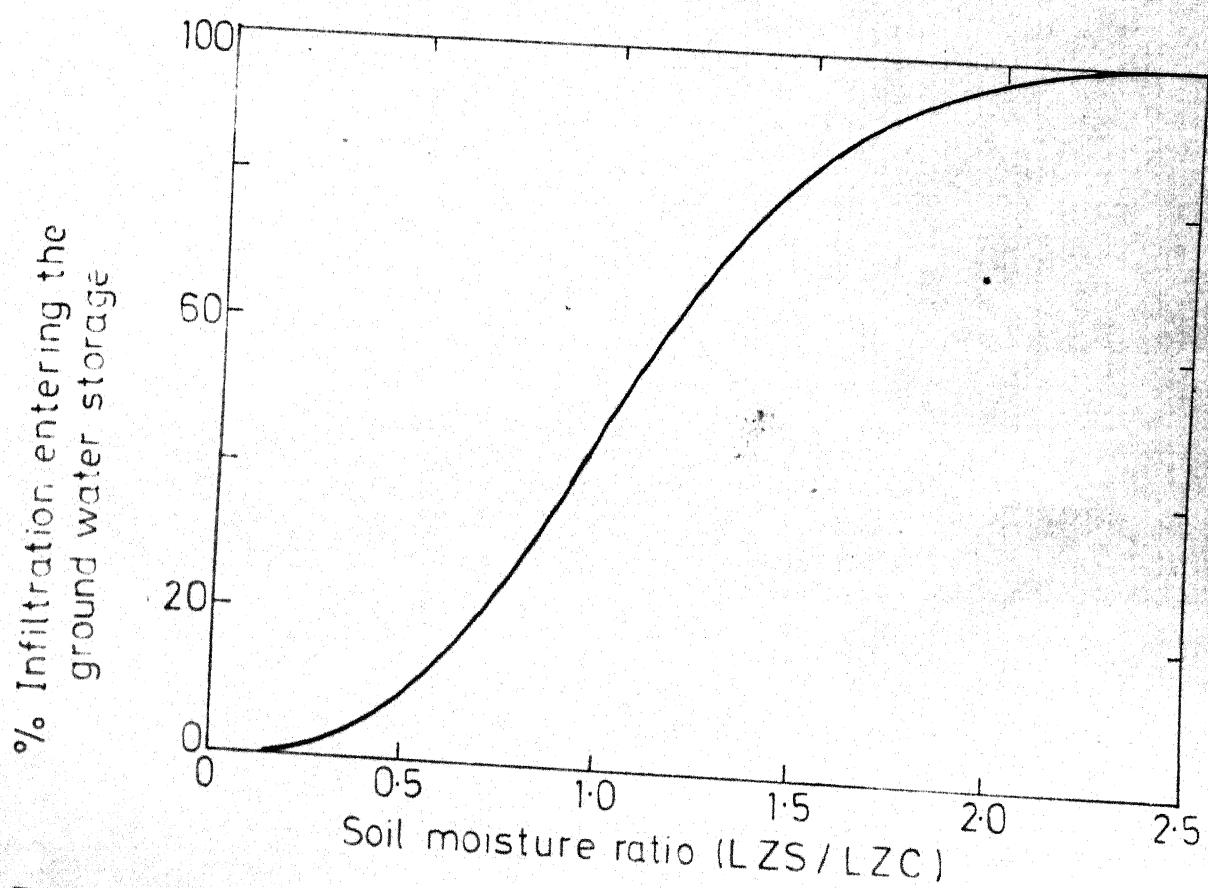


FIG.5 MODEL FOR ESTIMATING INFILTRATION ENTERING GROUNDWATER STORAGE

Maximum rate = $ETLF * LZS / LZC$

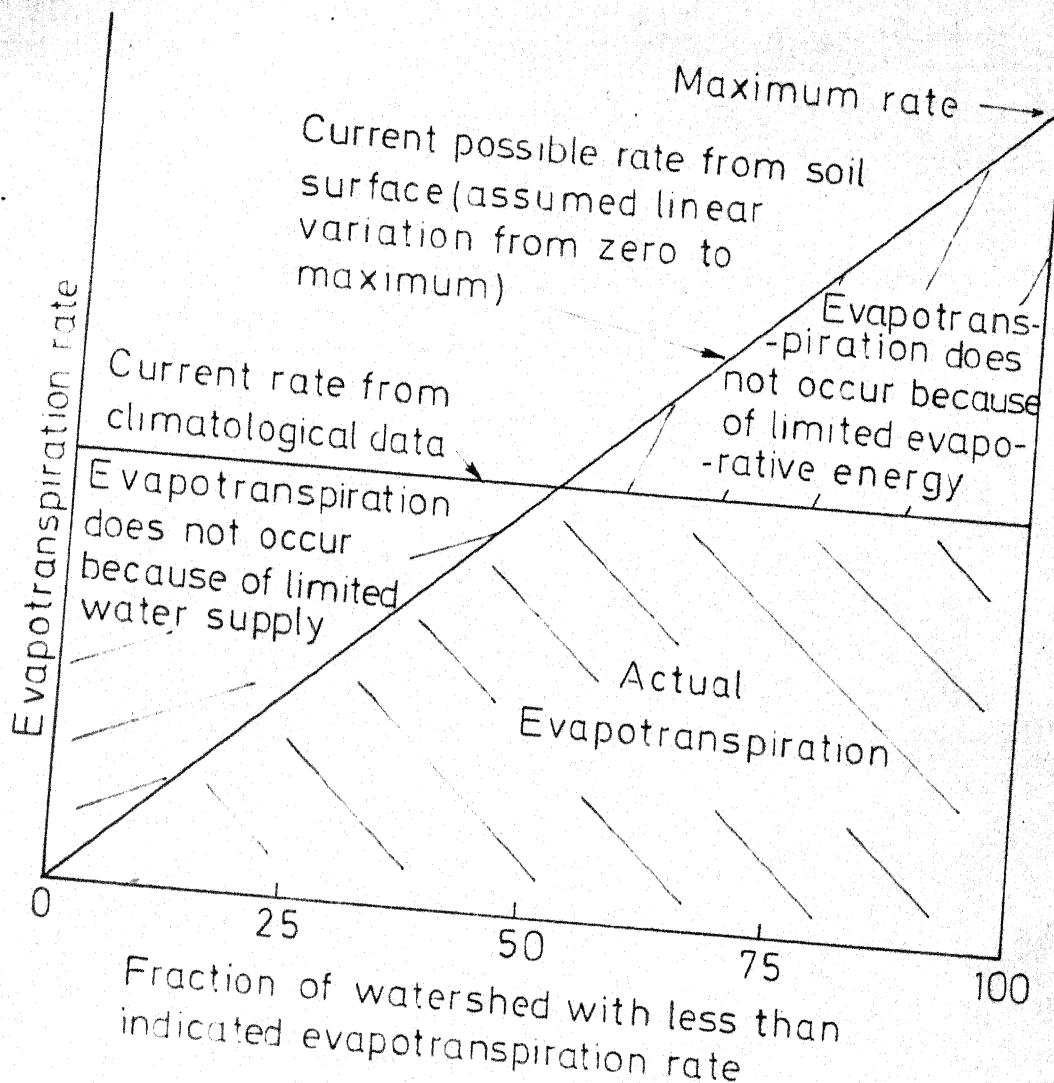


FIG. 6 MODEL FOR ESTIMATING ACTUAL EVAPOTRANSPIRATION

$$CMIR = \frac{\text{CONSTANT} * SIAM * BMIR}{\text{function (LZS/LZC)}}$$

$$SIAM = \text{function (SIAC)}$$

$$CIVM = BIVF * \text{function (LZS/LZC)}$$

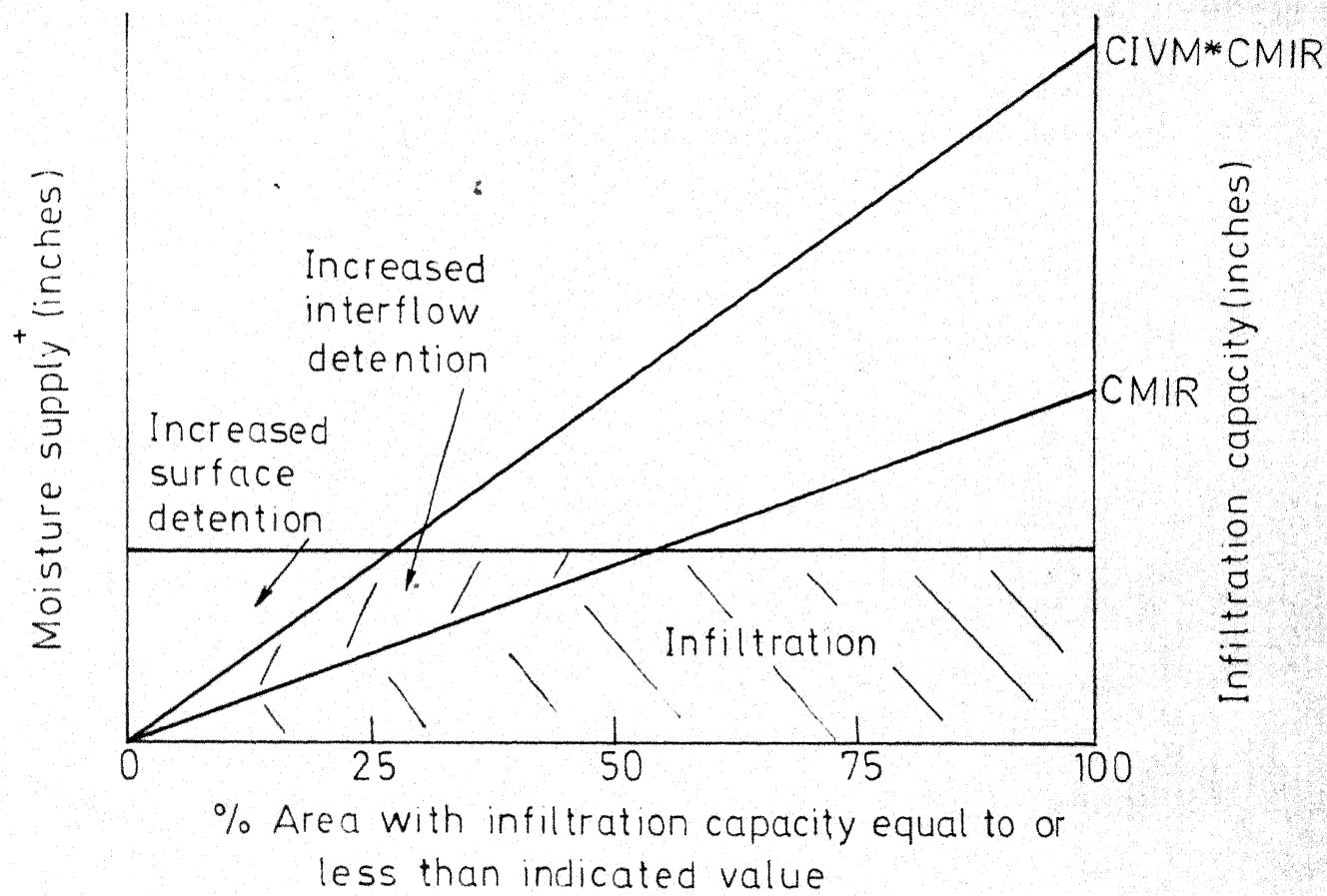


FIG. 7 MODEL FOR ESTIMATING INFILTRATION CAPACITY

- Rainfall passing through the upper zone plus holdover direct runoff

Increase in the value of BMIR reduces runoff during storm periods and later on increases runoff due to baseflow.

2.3.3 Surface Volume Parameter (BIVF):

It is an index controlling the time distribution and quantities of moisture entering the interflow. Increase in BIVF means more interflow by increasing the volume of CIVM. BIVF enters the streamflow simulation through the equation,

$$CIVM = BIVF \times 2^{(LZS/LZC)} \quad (\text{Ref. Fig. 7})$$

In order to check negative interflow CIVM must be ≥ 1 . It is determined by the subroutine SETBIV.

2.3.4 Channel Routing Parameters (CSR_X, FSR_X, NCTRI, CHCAP):

The subroutine SEMHRP estimates single best values for NCTRI and SRX for each of the 5 hydrographs (maximum 5) specified in the input data. Hydrograph NCTRI values are averaged in it to have single best value of NCTRI. The storage routing uses the equation,

$$O_2 = \bar{I} - SRX (\bar{I} - O_1)$$

where,

O_2 = routed outflow at the end of the time interval,

\bar{I} = average inflow during the time interval,

O_1 = outflow at the beginning of the time interval,

SRX = storage routing index.

SRX for low flows is denoted by CSR_X and SRX for flood flows is denoted by FSR_X.

SETSRP estimates two channel routing parameters CSRX, FSRX. When the synthesized streamflow is less than one-half of CHCAP, CSRX is used for routing. If the synthesized flow exceeds twice CHCAP, FSRX is used. When the synthesized flow is in between these two SRX is interpolated from the equation,

$$SRX = CSRX + (FSRX - CSRX) \times \left(\frac{Q - 0.5 \text{ CHCAP}}{1.5 \text{ CHCAP}} \right)^3$$

where,

Q is the synthesized streamflow.

2.4 Parameters Directly Estimated from the Watershed Characteristics:

1. AREA : Total area of the watershed in sq. miles,
2. FIMP : Fraction of watershed which is impervious and contributes its runoff directly into a stream.
3. FWTR : Fraction of total watershed covered by water surfaces,
4. VINTMR: Watershed interception volume storage capacity.

It is dependent upon the type and density of vegetative cover. A table to choose VINTMR is given below:

Table 1: Interception Values for Various Types of Cover. [4]

Watershed Cover	VINTMR (in)
Grass land	0.10
Moderate Forest cover	0.15
Heavy Forest cover	0.20

5. GWETF : It is a factor which when multiplied by the current rate of potential evaporation times the current ground water moisture storage gives an estimate of the current rate at which phreatophytes or swamp vegetation are drawing water from below the water table. For most of the basins this value is recommended to be zero [4].

6. SUBWF : Fraction of the moisture entering ground water storage which leaves the basin through subsurface flow not measured by the stream gauge. Generally its value is zero.

7. OFSS : Average slope (in ft/ft) of the overland flow surfaces perpendicular to the receiving channel. Mean value is taken from randomly selected group of points on a topographic map.

To find out the discharge from the over land flow by Chazy-Manning formula this OFSS is used in the following equation:

$$q = \frac{1,486}{n} (y^{5/3}) (S^{1/2})$$

where,

q = discharge in cfs/ft.

y = depth of flow in ft.

S = slope of the surface in ft/ft (OFSS)

8. OFSL : Mean overland flow length in ft.
It is the average distance that surface runoff in the watershed travels before reaching a channel. It is estimated as the reciprocal of twice the drainage density in sq.miles.

9. OFMN : Manning's roughness coefficient for overland flow on soil surfaces.

10. OFMNIS: Manning's roughness coefficient for overland flow over impervious surfaces.

11. CHCAP : Channel capacity - indexed to the outlet of basin:
CHCAP may be estimated from hydraulic analysis of the profile and cross-section of the stream channel.

12. DIV : This is the daily flowdiversion in cfs.
Diversions into the streams are considered as positive.

2.5 Earlier Studies Using OPSET:

The original work on the development of Watershed Model started in 1959 at Stanford University in USA. Its name was given as Stanford Watershed Model (SWM). This model was developed through the versions of I, II, III and IV and was programmed for the digital computer in SUBALGOL language. D.L. James translated the model III into FORTRAN (G-Level) and simultaneously incorporated most of the features of model IV. This model was named as Kentucky Watershed Model (KWM). The

University of Kentucky in the year 1970 developed a self calibrating model named as OPSET. Then this model was tested in 20 watersheds most of which are rural. Liou [8] and Ross [4] gave **full** reports of the application of this OPSET models in the U.S.A.

CHAPTER III

IMPLEMENTATION OF OPSET PROGRAM

3.1 Important Characteristics of the Program:

Important characteristics of the OPSET program are described below:

3.1.1 Program Size and Memory:

OPSET is a very lengthy program. In all, about 2400 cards are needed to be punched (excluding data). Available core memory in IBM 7044 at IIT Kanpur is less than 32 K words (1 word = 6 bytes in IBM 7044 system). This memory is not sufficient for the OPSET program. On IBM 7044 two compilers are available viz. (i) WATFOR, (ii) FORTRAN. WATFOR compiler gives an extensive diagnostic report but execution is slow. Due to memory problem, and even for debugging purpose, the whole program could not be run at a time. Main program and each subroutine had to be fed separately for correcting the syntax errors. This did not solve the purpose. FORTRAN compiler ignores the undefined variables and some erroneous part of the program.

All these difficulties are overcome through the IBM 370/155 computer system at IIT Madras. Here WATFIV compiler which has a memory upto 512K bytes, has been used to

debug the program. The total memory requirement with one set of data is 250 K (bytes) (1 word on IBM 370/155 system equals 4 bytes).

3.1.2 Computer Time:

In IBM 370/155 system through WATFIV compiler with one year's data and all the options, computer time needed is 15 minutes (compilation and execution). This implies that IBM 7044 will take more than one hour to do the same job, because IBM 370/155 compiles and executes respectively eight and four times faster compared to IBM 7044/1401 system at IIT/Kanpur.

3.1.3 TRIPS:

The program is divided into three portions. In execution of the first portion 6 variables are taken into account. In the execution of the second portion other variables are considered. In the final step all the variables are taken together to give final synthesis. Execution of each of these portions is called a TRIP. First TRIP gives the optimized values of LZC, BMIR, SUZC, ETLF, BUZC and SIAC. In addition to these values, two other parameters BIVF and LZS are also obtained. The second TRIP starts working when the above values are optimized. In this TRIP the other six parameters CSRX, FSRA, NCTRI, ChC.F, IFRC, BFRC are optimized. Here the parameter values of the first TRIP remain unaltered. In the third TRIP all the parameter values taken together give the final run with the simulated flow.

3.1.4 Language:

The OPSET program is written in FORTRAN language. In this program one subroutine READ is used to read data in free format. But this subroutine is not readily available with us. READ subroutine is therefore replaced by the formated READ statements.

3.2 Modifications:

1. Water Year.

In the United States the water year is counted from the 1st October of the one year to the 30th September of the next year. In this study it is taken from the month of June and is modified accordingly.

2. Evaporation Estimates:

In the test data of the OPSET program the evaporation is taken as the total evaporation in a year in inches and through the subroutine EVPDAY the daily values were calculated. In the present study the 10 daily average values of evaporation are used.

3. Flood Peaks:

The flood peaks data for the watershed, used in the present work, are not available. These values are, hence, interpolated from the stream flow data.

3.3 Addition and Alterations:

To take care of the nonavailability of some data prescribed for the program and to work with the program in a different computer system, some necessary changes have been made. These are described below:

1. All the evaporation data available in the study are in mm, but the OPSET program uses them in inches. Through the following statements the evaporation data in mm are changed to inches,

```
990 DPET (KRD) = 0.03937 *DPET (KRD)
```

```
991 DPET (KRD) = 0.03937 *DPET (KRD)
```

2. In the original program the following statements were used to read the recorded stream flow for one year.

```
118 DAY = 274
```

```
119 CALL REAL (DRSF (DAY))
```

```
CALL DAYNXT (DAY,DPY)
```

```
IF (DAY.NE.274) GO TO 119
```

It is necessary to change the above statements as in the present case the first day of a water year is the 152nd day of the calendar year and not the 274th day as stated in the program. The following statements are, therefore, introduced.

```
118 DAY = 274
```

```
READ 1000, (DUM(J),J = 1, DPY)
```

```
PRINT 1000, (DUM(J), J = 1, DPY)
```

```
ISEQ = 1
```

```

119  DRSPF (DAY) = DUM (ISEQ)
      ISEQ = ISEQ + 1
1000 FORMAT (13F6.1)
      CALL DAYNXT (DAY,DPY)
      IF (DAY.NE.(396-DPY)) GO TO 119

```

Here one dummy array DUM (J) is introduced. June 1 data is read as October 1 data. But these two dates differ in a year by 122 days, so the last line in the above statement 274 was replaced by 396-DPY.

3. The following statements are given in the main program of OPSET.

```

      DO 126 KRD = 1, NSGRD
      CALL READ (ISGRD)
126  CALL READ (DRSGP (ISGRD))
C      READ RECORDING RAIN GAGE HOURLY TOTALS
127  CALL READ (IVBG, YEAR, MONTH, DATE, CN)
C      PUNCH NO NUMBERS AFTER CN ON YEAR.EQ.98 CARD
      IF (YEAR.GE.98) GO TO 130
      HRF = 12 * (CN-1) + 1
      HRL = 12 * (CN-1) + 12
      DAY = MEDCY (MONTH) + DATE
      DO 128 HOUR = HRF, HRL
128  CALL READ (DRHP (DAY, HOUR))
      IF (DPY.NE.366.OR.MONTH.NE.2.OR.DATE.NE.29)GO TO 127

```

he above statements are changed to the following form for reasons mentioned above.

```
DO 126 KRD = 1, NSGRD
  READ 500, ISGRD
  READ 103, D, SGP (ISGRD)
126 DRSGP (ISGRD) = 0.03937 * DRSGP (ISGRD)
C  READ RECORDING R/T : GAGE HOURLY TOTALS
127 READ 500, IWBG, YEAR, MONTH, DATE, CN
C  PUNCH NO NUMBERS AFTER CN ON YEAR.EQ.98 CARD
  IF (YEAR.GT.98) GO TO 130
  HRF = 12 * (CN-1) + 1
  HRL = 12 * (CN-1) + 12
  DAY = MEDCY (MONTH) + DATE + 122
  IF (DAY.GT.DPY) DAY = DAY-DPY
  DO 2000 IJK = 1,12
  IF (DAY.GT.MEDCY (IJK)) GO TO 2000
  MONTH = IJK - 1
  GO TO 2050
2000 CONTINUE
2050 CONTINUE
  READ 1003, (DRHP (DAY,HOUR), HOUR = HRF, HRL)
  DO 1291 HOUR = HRF, HRL
1291 DRHP (DAY, HOUR) = 0.03937 * DRHP (DAY, HOUR)
  1003 FORMAT (12F 5.2)
  IF (DPY.NE.366.OR.MONTH.NE.2.OR.DATE.NE.29) GO TO 127
  DO 129 HOUR = HRF, HRL
Other alterations are mainly the changes in the FORMAT statements
and corrections of some printing errors in the OPSET program.
These are not mentioned here.
```

3.4 Time Spent on Collecting the Data and Implementing OPSFI:

The collection of relevant data, required for this program, constitute a large portion of the present work. Collection of data was started first in September 1974 and ultimately all the data could be collected from different places by the end of June 1975. Punching and correcting of the program started long before and ultimately in July 1975 the program was in perfect working condition. Only after that the work on the basic objective i.e. to use OPSFI for quantifying an Indian watershed, could be started. The work has been finally completed in October, 1975.

3.5 Factors Affecting the Efficiency of the Program:

1. Due to heterogeneous properties in the large watersheds it is very difficult to correlate the model parameters with watershed character. Watershed should be small enough to be homogeneous to minimise this difficulty.
2. The basin area should not be very large.
3. One run of OPSFI program takes a large amount of computer time and the computer expense is high. So for economic reasons it is desirable to use OPSFI for as few years as possible.
4. Two factors cause the variation of the parameter value for different water years. These are (i) differences in the pattern of deviation from year to year between gauge and watershed rainfalls and (ii) the differences in the pattern of the stream flows among the years.

CHAPTER IV

MODELING OF THE INDIAN BASIN AND THE RESULTS

4.1 Description of the Basin.

The basin is bordered by two canals on two sides (Fig. 8). The whole area of the basin is more or less flat. The climate is temperate during hot summer months from May to September. Thereafter air becomes drier and cooler every day. Hoarfrost is common in January and February. For few nights in the year the ground temperature falls below 0°C . Average mean monthly maximum and minimum temperatures vary between 60°F to 110°F and 35°F to 80°F respectively. Northeast part of the area receives more than 50 inches (1270 mm) of rainfall annually and the southwest part receives between 15 inches to 20 inches (381 mm to 508 mm). This area has been formed of alluvial deposits brought by the rivers. Some part is formed of clay, hard clay or clay mixed with kankar at the top.

4.2 Data Availability:

4.2.1 Measurable Watershed Data:

The test data for the year 1956 for the basin Woodcock at London, Ky. USA are available with us. The OPSWT project is first run with this set of data.

SCHILL 4
Scale 1:100,000
16 miles

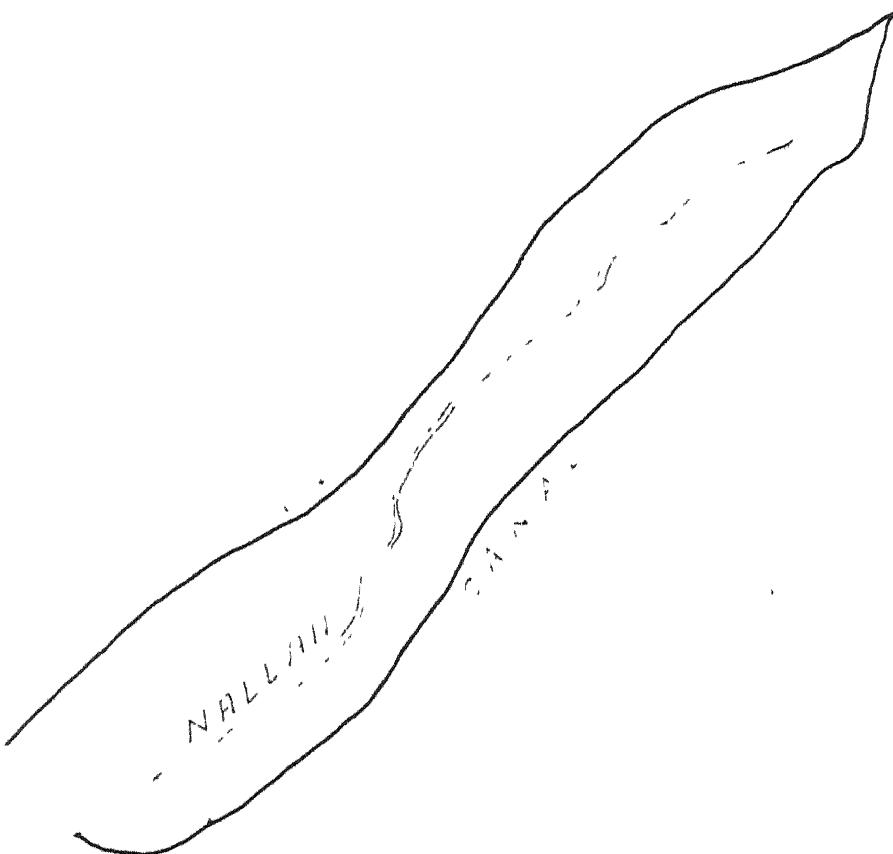


FIG. 8 PLAN OF THE BASIN

For the basin in this study the following data were measured as accurately as possible. The description of all these are presented in Chapter 2.

1. AREA:

The maximum value of this is 650 sq.miles. The streamflow data were obtained from different places along the stream in different years. The areas under considerations are also different.

2. FIMP:

The impervious part of the area is significantly small. This value has been taken as 0.0005.

3. FWTR:

There is no lakes or swamps inside the area. Hence this value is taken as zero.

4. VINTMR:

No forest is observed inside the basin. The value of VINTMR is taken from the Table 1 (Chapter II) as 0.10.

5. GWETF:

As a significant number of phreatophytes or swamps are not known to exist inside the area, this value is taken as zero.

6. SUBWF:

It is assumed that no water leaves the basin through sub-surfaceflow not measured by the stream gauge after entering into the ground water storage from the surface. This value is, therefore, taken as zero.

7. OFSS:

This is obtained from the average of the measurements at randomly selected points on the basin. 4 measurements are taken from which the average is calculated as described below:

<u>Measurement No.</u>	<u>Slope (in ft/mile)</u>
1	5
2	2
3	10
4	5

Average slope = 5.5 ft in 1 mile \approx 0.001 (= 0.1 percent)

8. OFSL:

OFSL can be estimated in different ways. In this study, it is represented as the reciprocal of twice the drainage density (obtained by dividing total lengths of all the streams within the watershed in miles with the total area of the watershed in sq.miles). The calculations is shown below.

Total area of the watershed = 650 sq.miles

Total length of all the streams
inside the watershed = 456.0 miles

Hence, the drainage density = $\frac{456.0}{650.0} = 0.70$

$$\text{OFSL} = \frac{1}{2 \times (\text{drainage density})} = \frac{1}{2 \times 0.70}$$

$$= 0.715 \text{ miles} = 3780 \text{ feet}$$

9. OFMN:

This value is taken from the table given below:

Table 2: Manning's Roughness Coefficient for Overland Flow for Various Surface Types [9]

Watershed Surface	Manning's n
Smooth Asphalt	0.013
Concrete (Trowel finish)	0.013
Rough Asphalt	0.016
Concrete (unfinished)	0.017
Smooth Earth	0.018
Firm Gravel	0.020
Cemented Rubble Masonry	0.025
Pasture (Short grass)	0.030
Pasture (High Grass)	0.035
Cultivated Area (Row Crops)	0.035
Cultivated Area (Field Crops)	0.040
Scattered Brush, Heavy Weeds	0.050
Light Brush and Trees (Winter)	0.050
Light Brush and Trees (Summer)	0.060
Dense Brush (Winter)	0.070
Dense Brush (Summer)	0.100
Heavy Timber	0.100

OFMN is taken for the basin as 0.035.

10. OFMNIS:

This value is also taken from the above table. For the present study it is taken as 0.018.

11. CHCAP:

The size of the channel at the gauging point may not be the size of the channel system as a whole. This can be measured from the hydraulic analysis of the profile and the cross section of the stream channel. Another method is to measure the gauge height of the bankful flow from the topographic map and then to read the CHCAP value directly from the stream gauge rating table. In this study CHCAP is indexed to the outlet of the basin and is taken as more than the measured maximum discharge.

12. DIV:

There is no flow diversion into or out of the stream and hence this value is taken as zero for the watershed in the study.

4.2.2 Meteorological Data:

1. Evaporation Data:

Pan evaporation data are available for one station near the basin. Evaporation data are recorded with mesh covered fixed point gauge, class 'A' pan evaporimeter. These values were in millimeters and are converted to inches by multiplying with 0.03937.

2. Pan Coefficients:

In this study these values are taken from the table given in Ref. 10. Starting from the month of January upto December, these values are 1.00, 1.00, 0.90, 0.75, 0.70, 0.70, 0.75, 0.90, 0.95, 0.90, 1.00 and 1.10.

3. Daily Hourly Precipitation Data:

These are recorded for one rain gauge station near the basin by a self recording rain gauge. These values are obtained again in mm and were converted to inches.

4. Daily Rainfall Data:

Daily rainfall data are also collected from an ordinary rain gauge station inside the basin. This raingauge is called in the program as the secondary rain gauge. The values obtained in mm were converted to inches.

Evaporation, hourly rainfall, daily rainfall and discharge data are collected for a few years. All the data are not available for the same place inside the basin. Two years data have been run but it is found that only in one particular year the rainfall data is in consistence with the discharge data.

5. Stream Flow Data.

This data is collected from the drainage nallah of the basin. Discharge data are not available for the basin for the same place for different years. These are available in different location in different years.

The basin is approximately rectangular in size and so the area covered by each stream gauge station is taken as linearly proportional to the distance of the stream gauge from the farthest end of the basin.

4.3 Simulation Analysis:

4.3.1 Initialization of Parameter Values:

OPSLI optimizes 13 parameters altogether. These are, LZC, BMIR, SUZC, ETLF, BUZC, SIAC, BIVF, BFRC, IFRC, CSEX, FSRX, ICFRI, CHCAP.

The following are the initial values of first six parameters inside the program.

Table 3: Initial Values of Parameters (Ref. 8, p. 33).

Starting Values

Parameter	Low	Middle	High
LZC	2.00	12.00	30.00
BMIR	0.20	1.20	4.00
SUZC	0.30	1.30	4.00
ETLF	0.05	0.25	0.60
BUZC	0.20	1.50	5.00
SIAC	0.30	0.90	4.00

For the consistent estimate of reasonable parameter values the medium starting values worked best because they give the best chance of being close to the final estimate and less chance to produce out-of-range adjusted values which may cause the program to stop before the optimum point is reached (Ref. 8, p. 34).

The initial values of the other parameters are as follows:

BIVF = 0.0 , FSRX = CSRX = SRX = 0.98

IFRC = 0.1 , BFRC = 0.9

4.3.2 Control Options:

1. CONOPT:

The OPSET program has the control options, expressed by CONOPT (1), CONOPT (2) and CONOPT (3).

- CONOPT (1) = 0, if daily evaporation data are used.
- = 1, if evaporation is read by 10 day period.
- = 2, if annual evaporation is read and the subroutine EVPDAY is used.
- CONOPT (2) = 0, if stream routing is to be done with 15 minutes time interval.
- = 1, if stream routing is to be done with hourly time interval.
- CONOPT (3) = 0, if no change in WSG or SGRT occur during the water year.
- = 1, if storage gauge was moved during the water year.

In the test data CONOPT (1), CONOPT (2) and CONOPT (3) were taken as 2, 0 and 1 respectively. For the Indian data in this study these are taken as 1, 1, and 0.

2. MNRC, NFTR, and NLTR:

These are another set of control options which are given below:

- MNRC = Minimum number of rough cycles to be made.
- NFTR = Number of first trip to be run for a given year.
- NLTR = Number of last trip to be run for a given year.

In all the cases, the values of MNRC, NFTR and NLTR are taken as 12, 1 and 3 respectively.

4 Output Parameters Using the Test Data:

Test data [4] have been run through the OPSET program. Comparative study of these output parameters and that given in the report [8] is given in the following table.

Table 4: Comparison of Obtained Parameter Values with those given in the Ref. 8.

Sl. No.	Parameter	Value given in the report	Value obtained from the program
1	LZC	11.14	11.76
2	BMIR	4.23	4.99
3	SUZC	0.65	0.65
4	STLF	0.15	0.15
5	SUZC	1.04	1.09
6	STIC	0.45	0.45
7	STIV	0.00	0.00
8	CSRX	0.935	0.939
9	PSRX	0.935	0.939
0	WCTRI	3.	3
1	IFRC	0.10	0.10
2	RFPC	0.895	0.894
3	CHICAP	100.0	100.0

In addition, the LZS value found in the report to be 58 and that obtained from the program is 8.83.

The synthesized and the recorded monthly flows and peak discharges are compared and shown in Table 5 and Table 6 respectively on the other pages.

Table 5: Comparison of Recorded and Synthesized Monthly flows for the Test Data.

Month	Recorded flow	Synthesized flow
October 1955	13.5	6.6
November 1955	11.8	8.2
December 1955	21.3	22.3
January 1956	63.9	107.4
February 1956	779.8	719.9
March 1956	469.4	451.4
April 1956	465.1	456.9
May 1956	160.5	78.6
June 1956	28.3	30.0
July 1956	202.3	253.2
August 1956	51.9	49.5
September 1956	26.5	117.5
Yearly Total	2294.3 sf ^d	2311.5 sf ^d

Table 6: Comparison of Recorded and Synthesized Hydrograph Peaks for the Test Data.

Date	Recorded		Synthesized	
	Peakflow (cfs)	Time	Peakflow (cfs)	Time
Feb. 17, 1956	506.0	11 PM	458.4	6.15 AM
Mar. 14, 1956	264.0	6 AM	213.1	6.00 AM
Apr. 6, 1956	133.0	10 AM	109.3	9.15 AM
Apr. 15, 1956	227.0	9 AM	355.7	8.30 AM

From the results shown in the preceding tables, it is evident that the model has been implemented with reliable accuracy. Here the lowest value of SSQM = 3.69

Obtaining fairly accurate results with the test data the model was then applied to an Indian basin. The following are the results obtained for the basin.

Table 7: Recorded and Synthesized Stream Flow For the Year 1971-72.

Months	Recorded	Synthesized
June	20.0	182.4
July	1306.0	2849.6
August	12460.0	9817.2
September	983.0	585.9
October	0.0	15.6
November	0.0	0.5
December	0.0	0.0
January	0.0	0.1
February	0.0	2.2
March	0.0	0.2
April	0.0	0.3
May	0.0	0.0
Yearly Total	14769.0 sf ^d	13454.0 sf ^d

Table 8: Comparison of Recorded and Synthesized Hydrograph Peaks for the Year 1971-72.

Date	Recorded		Synthesized	
	Peakflow (cfs)	Time	Peakflow (cfs)	Time
July 30	110.0	12 AM	121.0	0.15 AM
August 5	1950.0	12 AM	1898.8	1.00 AM
August 28	480.0	12 AM	122.8	0.15 AM

Lowest SSQM = 2.674

Table 9: Recorded and Synthesized Streamflow for the year 1969-70.

Months	Recorded Flow	Synthesized Flow
June 1969	0.0	6.0
July 1969	3416.0	634.5
August 1969	5850.0	11171.9
September 1969	2191.0	3464.9
October 1969	0.0	337.4
November 1969	0.0	41.0
December 1969	0.0	11.8
January 1970	0.0	8.8
February 1970	0.0	19.7
March 1970	0.0	12.0
April 1970	0.0	1.5
May 1970	0.0	1.4
Yearly Total	11457.0 sf ^d	15710.9 sf ^d

Table 10: Recorded and Synthesized Hydrograph Peaks for the Year 1969-70

Date	Recorded			Synthesized	
	Peak flow (cfs)	Time		Peak flow (cfs)	Time
29th July '69	425.0	12 AM		--	--
16th Aug. '69	340.0	12 AM		617.7	9.15 AM
13th Se t. '69	140.0	12 AM		123.2	0.15 AM

Lowest SSQM = 86.4999

Table 11: Optimum Parameter Values Obtained for an Indian Basin.

Parameter	Year 1969	Year 1971
LZC	12.0	2.3382
BMIR	--	11.6108
SUZC	1.3	3.0
STLF	0.092	0.1793
BUZC	1.5	0.2761
SIAC	4.0	3.6
BIVF	0.0	0.0
BFRG	0.9267	0.8884
IFRC	0.10	0.10
CSRX	0.9950	0.90
FSRX	0.9950	0.90
NCPRI	47	25
CHCAP	2500	2000

Table 12: Recorded Flows for Test Data at Wood Creek, London, Ky., USA, 1955-56.

Day	Oct	Nov	Dec	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sept
1	0.4	0.3	0.3	0.5	0.5	5.7	8.6	4.0	5.6	3.2	8.7	3.5
2	0.4	0.3	0.5	0.5	0.5	20.0	8.6	5.4	1.6	1.2	4.3	1.2
3	0.3	0.3	1.6	0.4	0.4	36.0	11.0	7.1	22.0	1.5	3.7	0.7
4	0.3	0.3	3.0	0.4	0.4	47.0	9.2	19.0	12.0	1.3	3.7	0.7
5	0.3	0.3	1.5	0.4	0.4	22.0	8.2	9.8	9.2	1.2	4.7	0.6
6	0.3	0.3	1.0	0.4	0.4	35.0	7.5	50.0	7.9	1.1	1.8	6.1
7	1.8	0.3	0.9	0.4	0.4	20.0	26.0	30.0	7.0	0.9	1.1	2.1
8	0.5	0.3	0.7	0.3	0.4	12.0	34.0	17.0	5.2	0.9	2.0	1.2
9	0.4	0.2	0.2	0.7	0.4	9.5	19.0	12.0	4.8	0.8	1.6	0.8
10	0.4	0.3	0.3	0.6	0.4	7.9	13.0	9.8	4.2	0.8	1.5	0.7
11	0.3	0.3	0.6	0.5	0.4	13.0	10.0	8.2	3.8	0.7	0.6	0.6
12	0.3	0.2	0.2	0.5	0.4	10.0	11.0	7.0	3.4	0.7	1.6	0.6
13	1.4	0.2	0.2	0.5	0.5	9.2	15.0	6.2	3.0	0.7	1.3	0.5
14	0.6	0.2	0.2	0.5	0.3	13.0	10.0	7.2	2.8	0.6	1.0	0.4
15	0.5	0.3	0.5	0.5	0.3	17.0	35.0	94.0	2.8	0.6	3.9	1.2
16	0.4	0.4	0.3	0.4	0.4	14.0	27.0	59.0	2.6	0.5	8.6	1.1
17	0.4	0.4	0.3	0.5	0.3	103.0	18.0	29.0	2.2	0.5	6.6	0.9
18	0.4	0.4	1.1	0.6	0.6	143.0	16.0	17.0	2.1	1.4	4.7	0.4
19	0.3	0.4	0.4	0.5	0.5	44.0	12.0	12.0	2.0	1.8	3.5	0.4
20	0.3	0.3	0.4	0.5	0.5	25.0	9.8	9.2	1.8	0.9	5.8	2.0
21	0.3	0.3	0.5	0.5	0.6	16.0	8.9	7.6	1.5	1.1	6.5	1.5
22	0.3	0.3	0.5	0.5	0.6	12.0	7.6	7.0	1.4	0.9	2.7	1.2
23	0.3	0.3	1.9	0.5	0.6	10.0	6.8	5.5	1.4	0.6	15.0	0.6
24	0.3	0.3	0.6	0.5	0.5	35.0	7.6	5.6	1.3	0.5	22.0	0.5
25	0.3	0.3	0.5	0.5	0.5	39.0	6.2	4.8	1.2	1.3	18.0	0.9
26	0.3	0.4	0.4	0.5	0.5	24.0	5.6	4.4	1.2	0.5	14.0	0.8
27	0.3	0.4	0.3	0.4	0.4	16.0	5.1	4.0	1.3	0.4	10.0	0.8
28	0.3	0.3	0.3	0.4	0.6	12.0	5.9	3.6	0.4	8.6	0.8	0.4
29	0.3	0.3	0.4	0.4	11.0	9.5	5.1	3.4	0.4	11.0	0.7	0.4
30	0.3	0.3	0.6	0.6	33.0	-	4.4	4.3	2.0	0.4	7.8	0.7
31	0.3	-	-	0.5	5.9	-	-	-	1.6	-	5.7	0.7

Table 13: Synthesized Flow for Test Data at Wood Creek, London, Ky, USA, 1955-56.

	Aug	Sept	Oct	Nov	Dec	Jan	Feb	Mar	Apr	May	Jun	July	Aug	Sept.	
1	0.3	0.0	0.4	0.2	0.1	7.6	7.4	3.3	2.6	3.7	17.2	5.1	3.5		
2	0.3	0.0	0.7	0.1	11.2	7.5	39.7	7.2	2.9	2.2	4.6	4.6	2.1		
3	0.2	0.0	2.1	0.1	31.7	8.2	14.8	5.2	2.5	1.8	4.0	3.6	1.5		
4	0.2	0.0	2.1	0.1	63.9	6.5	22.1	4.1	2.2	1.6	3.2	3.2	1.3		
5	0.2	0.0	1.6	0.1	14.1	6.5	5.8	3.7	2.0	1.9	1.6	2.8	1.3		
6	0.2	0.0	1.4	0.1	43.6	5.9	108.9	3.3	1.8	1.6	2.5	17.3	63.7		
7	0.8	0.0	1.3	0.1	11.0	79.6	10.0	2.9	1.6	1.5	2.2	2.0	2.9		
8	0.3	0.0	1.2	0.1	9.8	10.6	7.2	2.6	1.4	1.3	1.3	1.3	3.2		
9	0.2	0.0	1.2	0.1	8.7	6.6	6.4	2.3	1.2	1.2	1.2	1.2	2.9		
10	0.2	0.0	1.1	0.1	7.8	5.9	5.7	2.0	1.1	1.1	1.1	1.1	2.5		
11	0.2	0.0	0.9	0.1	26.5	9.7	5.1	1.8	0.9	1.0	1.0	1.0	2.5		
12	0.2	0.0	0.9	0.1	8.7	24.5	4.6	1.6	0.8	1.1	1.4	1.4	2.0		
13	0.8	0.0	0.9	0.1	8.0	24.6	4.1	1.4	0.7	0.7	1.3	1.3	1.8		
14	0.4	0.0	0.9	0.1	23.0	72.8	6.9	1.3	0.8	0.8	5.5	5.5	1.6		
15	0.3	0.2	0.2	0.1	10.8	10.8	115.3	1.3	0.6	0.6	6.0	6.0	1.5		
16	0.3	0.1	0.5	0.1	11.3	25.5	22.2	1.1	0.6	0.6	7.4	7.4	1.5		
17	0.3	0.5	0.5	0.1	158.8	10.3	9.2	0.9	0.4	0.4	20.0	20.0	0.7		
18	0.2	0.1	0.7	0.1	72.4	14.6	8.2	0.8	0.5	0.5	6.4	6.4	0.6		
19	0.2	0.9	0.5	0.5	15.5	9.2	7.3	0.7	0.3	0.3	4.8	4.8	0.9		
20	0.2	0.2	0.5	0.7	13.6	8.2	6.5	0.6	0.4	0.4	8.5	8.5	0.8		
21	0.1	0.5	0.4	0.6	12.2	7.3	5.8	0.5	1.0	1.0	5.4	5.4	0.7		
22	0.1	0.4	0.4	0.6	10.9	6.5	5.7	0.5	0.4	0.4	4.6	4.6	0.4		
23	0.1	0.1	1.3	0.3	9.9	5.9	5.7	0.4	0.4	0.4	15.6	15.6	0.4		
24	0.1	0.1	0.7	0.3	6.9	13.9	5.1	0.5	0.4	0.4	77.4	77.4	0.3		
25	0.1	0.7	0.7	0.3	23.9	5.7	4.5	0.4	0.4	0.4	12.7	12.7	0.3		
26	0.1	0.6	0.2	0.5	10.8	5.1	4.0	0.3	0.3	0.3	15.6	15.6	0.2		
27	0.1	0.5	0.2	0.5	9.7	4.5	3.6	0.3	0.2	0.2	8.4	8.4	0.3		
28	0.1	0.5	0.2	0.7	8.8	13.3	3.2	1.4	0.2	0.2	7.3	7.3	0.4		
29	0.0	0.4	0.2	0.2	75.4	7.9	5.3	3.1	19.9	19.9	0.2	0.2	0.2		
30	0.0	0.4	0.2	0.2	18.6	4.2	2.7	4.6	4.6	4.6	6.4	6.4	0.7		
31	0.0	-	0.2	0.4	-	3.7	-	2.5	-	2.5	-	5.7	5.7	0.6	

Table 15: Synthesized Daily Flows (Indian Basin, 1971-72)
(cfs)

DAY	JUNE	JULY	AUGUST	SEPT.	OCT.	NOV.
1	0.0	70.8	101.0	70.2	2.3	0.1
2	0.0	90.6	841.9	62.3	2.0	0.1
3	0.0	185.8	1277.3	59.5	1.8	0.1
4	0.0	194.6	1115.2	55.9	1.6	0.1
5	0.0	208.3	724.9	49.6	1.4	0.0
6	0.0	207.0	591.2	44.1	1.3	0.0
7	0.0	192.5	517.5	39.2	1.1	0.0
8	0.0	175.5	458.5	34.8	1.0	0.0
9	0.0	158.1	407.0	30.9	0.9	0.0
10	0.0	141.4	361.5	27.5	0.8	0.0
11	0.0	125.9	321.0	24.4	0.7	0.0
12	0.0	111.9	285.1	21.7	0.6	0.0
13	0.0	99.4	253.4	19.2	0.6	0.0
14	0.0	88.3	224.9	17.1	0.6	0.0
15	0.0	78.4	199.7	15.2	0.5	0.0
16	0.0	69.6	177.4	13.5	0.4	0.0
17	0.0	61.8	190.9	12.0	0.4	0.0
18	0.0	71.6	174.1	10.6	0.4	0.0
19	0.0	79.5	154.6	9.4	0.3	0.0
20	0.0	70.6	137.3	8.4	0.3	0.0
21	0.0	62.6	121.9	7.4	0.2	0.0
22	0.0	55.6	108.3	6.6	0.2	0.0
23	0.0	49.4	96.2	5.9	0.2	0.0
24	0.1	70.7	85.4	5.2	0.2	0.0
25	2.3	69.0	75.8	4.6	0.2	0.0
26	20.8	68.8	79.9	4.1	0.1	0.0
27	30.7	65.9	113.9	3.7	0.1	0.0
28	29.2	58.5	112.8	3.2	0.1	0.0
29	28.4	94.8	100.2	2.9	0.1	0.0
30	27.8	128.1	89.0	2.6	0.1	0.0
31	-	113.8	79.0	-	0.1	-

Flows in the other months are all zero.

Table 16: Recorded Daily Flows (Indian Basin, 1969-70).
(cfs)

DAY	JUNE	JULY	AUGUST	SEPT.	OCT.	NOV.
1	0.0	0.0	305.0	102.0	0.0	0.0
2	0.0	0.0	305.0	102.0	0.0	0.0
3	0.0	0.0	283.0	100.0	0.0	0.0
4	0.0	0.0	262.0	100.0	0.0	0.0
5	0.0	0.0	242.0	100.0	0.0	0.0
6	0.0	0.0	242.0	90.0	0.0	0.0
7	0.0	0.0	180.0	85.0	0.0	0.0
8	0.0	0.0	150.0	65.0	0.0	0.0
9	0.0	45.0	120.0	65.0	0.0	0.0
10	0.0	40.0	118.0	110.0	0.0	0.0
11	0.0	45.0	115.0	115.0	0.0	0.0
12	0.0	50.0	115.0	120.0	0.0	0.0
13	0.0	55.0	118.0	134.0	0.0	0.0
14	0.0	58.0	134.0	125.0	0.0	0.0
15	0.0	60.0	140.0	110.0	0.0	0.0
16	0.0	62.0	327.0	100.0	0.0	0.0
17	0.0	66.0	305.0	100.0	0.0	0.0
18	0.0	66.0	305.0	90.0	0.0	0.0
19	0.0	70.0	262.0	92.0	0.0	0.0
20	0.0	66.0	184.0	86.0	0.0	0.0
21	0.0	60.0	262.0	60.0	0.0	0.0
22	0.0	50.0	184.0	45.0	0.0	0.0
23	0.0	66.0	150.0	40.0	0.0	0.0
24	0.0	66.0	167.0	30.0	0.0	0.0
25	0.0	150.0	160.0	25.0	0.0	0.0
26	0.0	327.0	131.0	0.0	0.0	0.0
27	0.0	350.0	134.0	0.0	0.0	0.0
28	0.0	424.0	120.0	0.0	0.0	0.0
29	0.0	450.0	115.0	0.0	0.0	0.0
30	0.0	400.0	110.0	0.0	0.0	0.0
31	-	380.0	102.0	-	0.0	-

Flows in the other months are all zero.

Table 17: Synthesized Daily Flows (Indian Basin, 1969-70)
(cfs)

DAY	JUNE	JULY	AUGUST	SEPT.	OCT.	NOV.
1	0.0	1.2	39.9	270.0	30.3	0.0
2	0.0	1.2	77.6	250.2	28.1	0.0
3	0.0	1.1	96.0	231.8	26.0	0.0
4	0.0	1.0	215.9	214.8	24.1	0.2
5	0.0	0.9	258.4	199.0	22.4	0.0
6	0.0	0.9	241.0	184.4	20.7	0.0
7	0.0	0.8	223.3	171.0	19.2	0.0
8	0.0	2.7	218.2	158.7	17.8	0.0
9	0.0	3.2	295.4	147.0	16.5	0.0
10	0.0	3.0	330.5	136.2	15.3	0.0
11	0.0	3.1	307.9	132.3	14.1	0.0
12	0.0	3.3	286.0	127.8	13.1	0.0
13	0.0	5.4	265.2	118.4	12.1	0.0
14	0.0	6.7	246.0	109.7	11.3	0.0
15	0.0	6.6	262.0	101.7	10.4	0.0
16	0.0	7.4	577.6	94.0	9.7	0.0
17	0.0	10.2	597.7	87.3	8.9	0.0
18	0.0	9.2	613.8	81.5	8.3	0.0
19	0.0	8.8	627.2	75.7	7.7	0.0
20	0.0	8.2	582.7	70.2	7.1	0.0
21	0.0	20.0	539.9	65.0	6.6	0.0
22	0.0	26.9	548.5	60.3	6.1	0.0
23	0.0	40.4	534.4	55.8	5.6	0.0
24	0.0	72.9	495.5	51.7	5.3	0.0
25	0.0	67.7	460.6	47.9	4.9	0.0
26	0.1	62.7	426.7	44.4	4.5	0.0
27	0.2	58.1	395.4	41.0	4.2	0.0
28	1.6	49.9	366.4	38.1	3.9	0.0
29	1.5	53.8	339.5	35.3	3.6	0.0
30	1.3	46.4	314.5	32.7	0.0	0.0
31	-	43.0	291.4	-	0.0	-

Flows in the other months are very negligible.

CHAPTER V

DISCUSSIONS AND CONCLUSIONS

5.1 Discussion of the Results:

5.1.1 SSQM:

SSQM is the sum of squares of deviations for 11 months and it is used to indicate which set of volume parameter values gives the best synthesis of flow volumes. The first month of the water year is excluded from this least square term because the flows in the first month are too dependent on unknown initial conditions. The final value of SSQM gives a reliable indication as to how best the parameters are synthesized with respect to the input data.

5.1.2 Test Data:

After going through the results from the test data, it is observed that the optimized 13 output parameters are closely comparable with the reported values [8]. The recorded daily stream-flow and the synthetic flow, as shown in the Tables 12, 13 are also comparable. In addition, from Table 6, it is clear that peak hydrograph values also matched well. Hence the OPSET program is well implemented, in the computer system available with us. Monthly flows are compared in Fig. 9.

5.1.3 Indian Data:

After analysing the output for 2 years for Indian data, the following observations can be made.

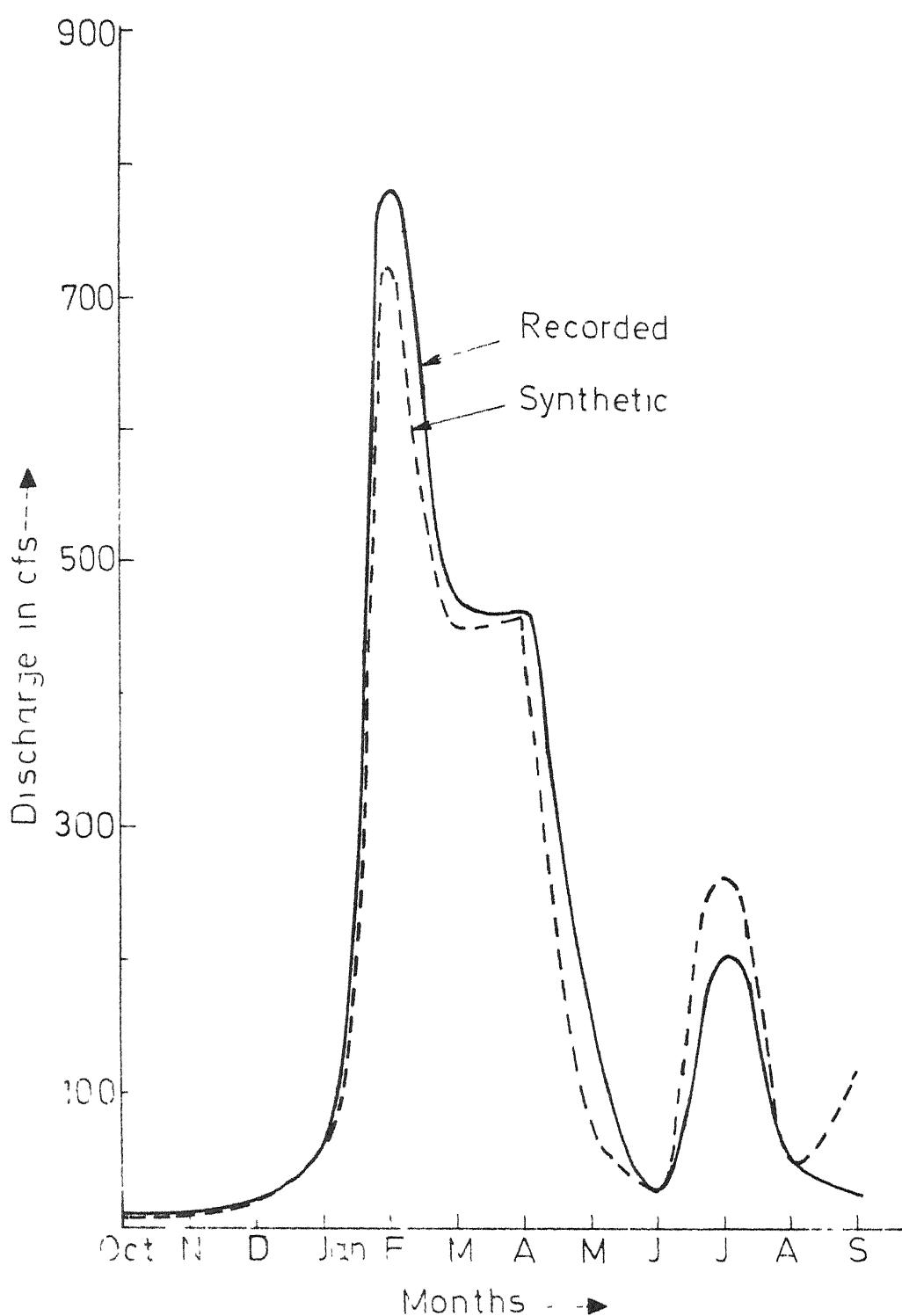


FIG. 1 TEST DATA - WOOD CREEK, USA, WATER YEAR 1956
(MONTHLY FLOW)

Year 1969-70:

From the table 16,17, it is observed that the daily recorded stream flow did not match with the daily synthesized stream flow. Similarly the monthly flows and the yearly totals of recorded and synthesized stream flows have wide gap. Peak hydrograph values between the observed and the synthetic cannot be compared. Hence the parameter values obtained on the basis of this year data are not correct. SSQM value in this year is high. The daily recorded and synthesized flows are shown in Fig. 11.

Year 1971-72:

From the Figure 10 and the Tables 14 and 15 of the daily stream flow hydrograph and the monthly totals respectively for the observed and the synthesized values, it is seen that these two matched well. Again the recorded hydrograph peak values can be compared with the synthesized hydrograph peak values. The SSQM value is very low in this year. Hence the parameter values for the year 1971-72 are taken as the optimum parameters for the basin. These values are:

LZC = 2.338 ,	BNIR = 11.610 ,	SUZC = 3.00 ,
ETLF = 0.1793 ,	BUZC = 0.2761 ,	SIAC = 3.60 ,
BIVF = 0.0 ,	BFRC = 0.8884 ,	IFRC = 0.10 ,
CSRX = 0.90 ,	FSRA = 0.90 ,	NCTRI = 25 and
CHCAF = 2000 .		

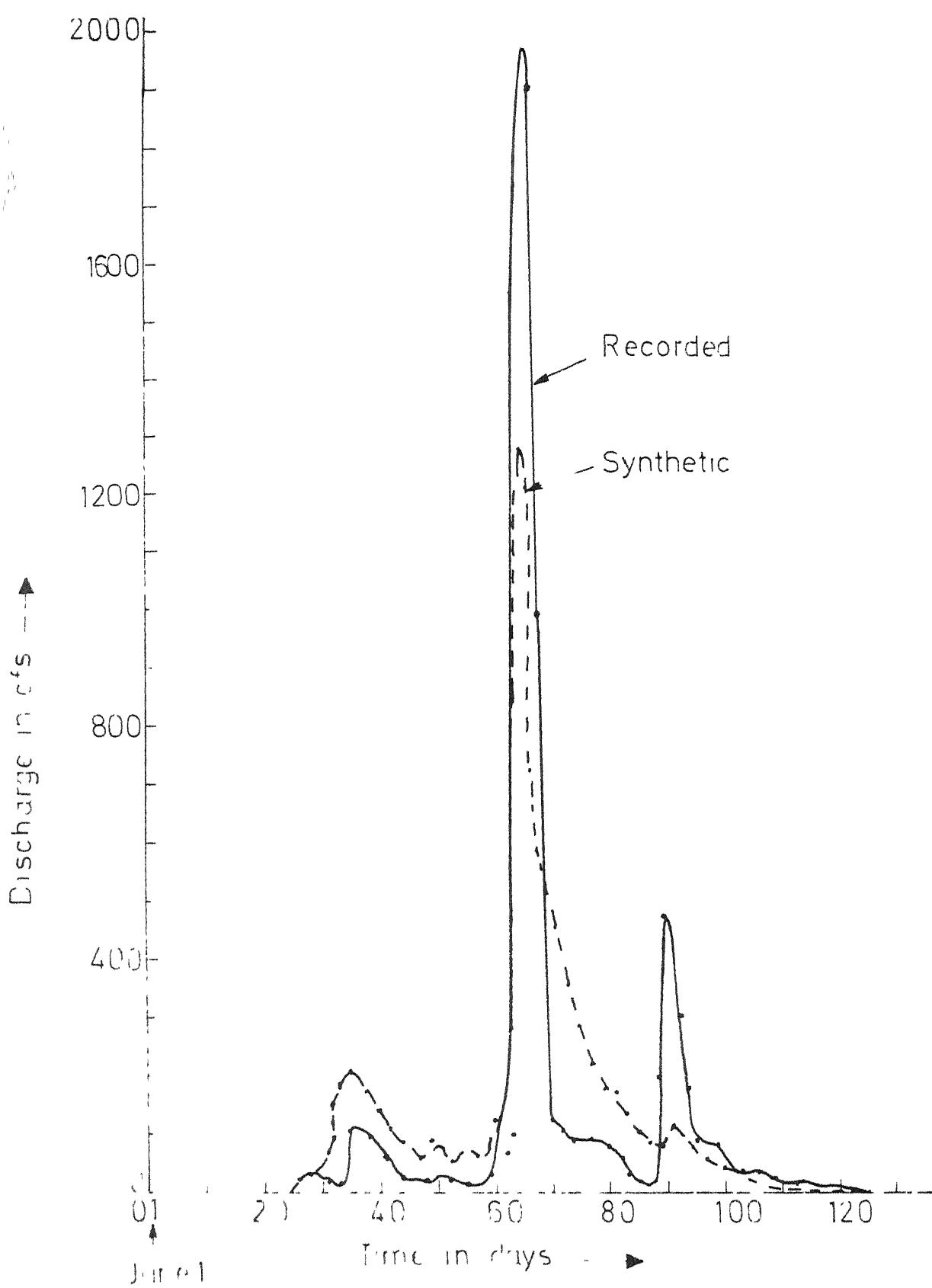


FIG.1.0 RECORDED AND SYNTHESIZED DAILY FLOWS OF
INDIAN BASIN WATER YEAR 1971-72

It is clear that the results obtained for the Indian basin are not really accurate. Also, the results from the different years are not equal to each other. This is basically due to the following factors.

- (i) All the rain gauge stations whose recorded hourly and daily rainfalls, used in the present work, are not situated within the basin. So the rainfall pattern fed to the program is different from the actual pattern.
- (ii) Streamflow records are not reliable. Some values are missing and some are not observed.
- (iii) Topographic sheet for the area is not available.
- (iv) Watershed area is large.
- (v) Unlike the test data for rainfall, the rainfall in the basin under study occur mainly for 3 months in a year.
- (vi) The peak flow discharge data are not available.
- (vii) The streamflow gauging points are different in different years.

However, the parameter values obtained from the data for the year 1971 are the better of the two. They can be taken as very good approximation for the parameters of the watershed. This can be inferred from the following reasons:

- (i) In the year 1971 the rainfall and streamflow patterns are comparable.
- (ii) SSQM value is very low in this year.

(iii) With days passing, the data availability also is more. The data which are not available for the preceding years are available for the year 1971. Hence, the results also can be considered better.

5.2 Conclusions and Scope of Further Studies.

Through the OPSET program very good prediction of annual streamflow and fairly good prediction of monthly streamflow are obtained. Flood hydrographs are simulated less accurately.

Natural phenomena being simulated are frequently too complex to permit an intricate analysis and to simplify these further research is necessary.

Measurable watershed parameters should be measured very accurately and the proper authorities should be well informed in advance to keep the necessary data for further studies. Peak flows are very much needed for estimating the channel routing parameters.

OPSET program should be applied in different watersheds varying in sizes, locations and climatological conditions.

Effect in the changes in the watershed such as urbanization and agricultural land use should also be studied through this program.

For larger watersheds, it is better to subdivide the total area into parts, each with its own set- of parameters,

and combine these results by stream routing.

Further detailed studies should be made to find out the ground water re-charge through this model.

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APPENDIX

COMPARISON OF ANNUAL RUNOFF CALCULATED BY OPSET METHOD WITH THAT BY AN APPROXIMATE METHOD

TEST DATA: Year 1956, Wood Creek, Ky, USA.

Approximate Method:

$$\text{Runoff} = \text{Rainfall} - \text{Evapotranspiration}$$

In this year of test data

$$\text{Rainfall} = 53.86 \text{ inches.}$$

$$\text{Evapotranspiration (Potential)} = 35.785 \text{ inches.}$$

$$\text{Annual Runoff} = (53.86 - 35.785) = 18.075 \text{ inches.}$$

$$\text{Total area of the watershed} = 3.89 \text{ square miles.}$$

$$\text{Annual Run off} = \frac{3.89 \times (5280)^2}{12 \times 86400} \times 18.075 = 1920 \text{ sfc}$$

Net evapotranspiration calculated by OPSET program is 29.094 inches.

$$\text{Hence Annual Runoff} = (53.86 - 29.094) = 24.766 \text{ inches.}$$

$$= \frac{3.89 \times (5280)^2}{12 \times 86400} \times 24.766 = 2582 \text{ sfd}$$

Annual Runoff calculated by OPSET = 2311.5 sfd.

It seems that the net evapotranspiration value gives the better result in Approximate method.

For Indian data the total annual evapotranspiration values always exceeded the net precipitation in the year and hence the above method could not be used. Khosla's method could not be applied due to non-availability of the value of the constant for the catchment under consideration.

SHM00000
SHM00010
SHM00020
SHM00030
SHM00040
SHM00050
SHM00060
SHM00070
SHM00080
SHM00090
SHM00100
SHM00110
SHM00120
SHM00130
SHM00140
SHM00150
SHM00160
SHM00170
SHM00180
SHM00190
SHM00200
SHM00210
SHM00220
SHM00230
SHM00240
SHM00250
SHM00260
SHM00270
SHM00280
SHM00290
SHM00300
SHM00310
SHM00320
SHM00330
SHM00340
SHM00350
SHM00360
SHM00370
SHM00380
SHM00390
SHM00400
SHM00410
SHM00420
SHM00430
SHM00440
SHM00450
SHM00460
SHM00470
SHM00480
SHM00490
SHM00500
SHM00510
SHM00520
SHM00530

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LISTING OF THE COMPUTER PROGRAMME
.....

C COMPUTER PROGRAM ON HYDROLOGIC SIMULATION
C OPSET
C A SELF-CALIBRATING VERSION OF THE STANFORD WATERSHEDMODEL
C BASIC LOGIC OF INNER LOOP BASED ON STANFORD WATERSHED MODELS III AND I
C VERSION OF NOVEMBER 12, 1970
C *****
C *
C *
C IMPLEMENTED FOR THE SYSTEMS IBM 7044 AND IBM 370/155
C *
C *
C *****
C IN THIS STUDY WATER YEAR STARTS FROM JUNE 1
C DAY = 274 IN THE PROGRAMME IS TAKEN FOR JUNE 1
C MONTHS IN THE TABULATED RECORDED AND SYNTHESIZED FLOWS ARE CHANGED
C IN THE NEW FORMAT ALSO DATES SHOULD BE READ AS FOLLOWS
C ***** JUNE 1 AS JUNE 1 *****
C ***** JULY 1 AS JULY 2 *****
C ***** AUG 1 AS AUG 2 *****
C ***** SEPT 1 AS SEPT 1 *****
C ***** OCT 1 AS OCT 2 *****
C ***** NOV 1 AS OCT 30 *****
C ***** DEC 1 AS NOV 30 *****
C ***** JAN 1 AS DEC 30 *****
C ***** FEB 1 AS JAN 30 *****
C ***** MAR 1 AS MAR 1 *****
C ***** APR 1 AS APR 1 *****
C ***** MAY 1 AS MAY 2 *****
.....

DIMENSION BTRI(99), CONOPT(5), CTRI(99), DRGPM(366), DRHP(366,24),
 1 DRSGP(366), DPET(366), DRSF(366), DSSF(366), EMGWS(12),
 2 EMIFS(12), EMLZS(12), EMSIAM(12), EMUZC(12), EMUZS(12),
 3 EPCM(12), HBF(5), IDYB(5), IDYE(5), IHRB(5), IHRE(5), KPSH(5),
 4 LSHA(5), MEDCY(12), MEDWY(12), RHPD(5), RHPF(5), RHPH(5)
 DIMENSION RSBBF(20), RSBDF(20), RSBIF(20), SBFRS(3,20)
 6 , THSF(24), TITLE(20), TMBF(12), TMIF(12), TMNET(12), TMOF(12),
 7 TMPET(12), TMPREC(12), TMRTF(12), TMSE(12), TMSTF(12),
 8 TMSTFI(12), UHFA(99), XMPFT(12), DUM(368), SIFRS(3,20), SSR(5,170)
 LOGICAL LBMIR, LBUZC, LETLF, LLZC, LNPR, LRC, LSHA, LSHP
 INTEGER CN, CONOPT, DATE, DAY, DPY, EHSGD, HOUR, HRF, HRL, PDAY,
 1 PRD, RHPD, RHPH, RSBDF, SGMD, SGRT, SGRT2, TRIP, YEAR, YR1,
 2 YR2
 REAL IPPRC, IFRC, IFRL, IFS, LZC, LZRX, LZSR, MNRD, NHPT
 DATA MEDCY/ 0, 31, 59, 90, 120, 151, 181, 212, 243, 273, 304, 334 /
 DATA MEDWY/ 304, 334, 365, 31, 59, 90, 120, 151, 181, 212, 243, 273 /
 C SPECIFY NUMBER OF STATION-YEARS INCLUDED IN COMPUTER RUN
 NSYC = 0
 READ 500, NSYT
 500 FORMAT(5I5)
 100 NSYC = NSYC + 1
 C READ TITLE TO COMPUTER RUN
 READ (5,1) TITLE
 1 FORMAT(20A4)
 C READ CONTROL OPTIONS
 READ 101, (CONOPT(I), I=1,3)
 101 FORMAT(3I2)
 READ 101, MNRC, NFTR, NLTR
 C READ BASIC TIME-AREA HISTOGRAM
 DO 102 KIA = 1, 99
 BTRI(KIA) = 0.0
 102 UHFA(KIA) = 0.0
 READ 500, NBTRI
 READ 103, (BTRI(J), J=1, NBTRI)
 103 FORMAT(8F10.5)
 C SET INITIAL CONDITIONS
 IFT = 1
 LRC=.TRUE.
 LLZC =.FALSE.
 LBUZC =.FALSE.
 LBMIR =.FALSE.
 LETLF = .FALSE.
 LNPR = .FALSE.
 IF(CONOPT(2) .EQ. 0 .AND. NBTRI.LE. 6) LNPR= .TRUE.
 KRC= 1
 KBRC = 0
 KFFC = 0
 SSSQM= 950.0
 SGRT = 0
 C READ FIXED PARAMETERS
 104 READ 103, RMPF, CHCAP
 READ 103, RGPME, AREA, FIMP, FWTR
 READ 103, VINTMR, SUBWF, GWETF, OFSS, OFMN, OFMNIS, OFSL, DIV

C CALCULATE CONSTANTS SET BY FIXED PARAMETERS SHM01080
 FPER = 1.0 -FIMP-FWTR SHM01090
 IF(FPER .GT. 0.01) GOTO 105 SHM01100
 TPLR = 100.0 SHM01110
 FPER = 0.01 SHM01120
 GO TO 106 SHM01130
 105 TPLR = (1.0-FWTR)/FPER SHM01140
 106 VWIN = 26.8888*AREA SHM01150
 WCFS = 24.0*VWIN SHM01160
 RHFMC = 0.025/WCFS SHM01170
 SSRT = SQRT(OFSS) SHM01180
 OFRF = 1020.0*SSRT/(OFMN *OFSL) SHM01190
 OFRFIS=1020.0*SSRT/(OFMNIS*OFSL) SHM01200
 EQDF= 0.00982*((OFMN*OFSL/SSRT)**0.6) SHM01210
 EQDFIS = 0.00982*((DFMNIS*OFSL/SSRT)**0.6) SHM01220
 RGPM =RGPMB SHM01230
 C READ WATER YEAR SHM01240
 READ 101,YR1,YR2 SHM01250
 DPY = 365 SHM01260
 IF(MOD(YR2,4) .EQ. 0) DPY = 366 SHM01270
 C READ EVAPORATION DATA SHM01280
 IF(CONOPT(1) .NE. 1) GO TO 111 SHM01290
 READ 103,(DPET(KRD),KRD=274,360,10) SHM01300
 DO 990 KRD=274,360,10 SHM01310
 990 DPET(KRD) = 0.03937*DPET(KRD) SHM01320
 READ 103,(DPET(KRD),KRD=1,273,10) SHM01330
 PRINT 103, (DPET(KRD), KRD= 1,273,10) SHM01340
 DO 991 KRD = 1,273,10 SHM01350
 991 DPET(KRD) = 0.03937*DPET(KRD) SHM01360
 DO 110 IDAY2=1,9 SHM01370
 DO 109 IDAY1= 274,360,10 SHM01380
 DAY= IDAY1 +IDAY2 SHM01390
 109 DPET(DAY)= DPET(IDAY1) SHM01400
 DO 110 IDAY1=1,273,10 SHM01410
 DAY=IDAY1+ IDAY2 SHM01420
 IF(DAY.GT. 273) GO TO 110 SHM01430
 DPET(DAY) =DPET(IDAY1) SHM01440
 110 CONTINUE SHM01450
 DPET(366)=DPET(59) SHM01460
 DPET(365) = DPET(363) SHM01470
 DPET(364)= DPET(363) SHM01480
 GO TO 113 SHM01490
 111 IF(CONOPT(1) .EQ. 2) GO TO 116 SHM01500
 DAY =274 SHM01510
 112 READ 103,DPET(DAY) SHM01520
 IF(DAY .EQ. 273) GO TO 113 SHM01530
 CALL DAYNXT(DAY, DPY) SHM01540
 GO TO 112 SHM01550
 113 READ 103,(EPCM(MONTH),MONTH=1,12) SHM01560
 PRINT 103, (EPCM(MONTH) , MONTH=1,12) SHM01570
 EPAET=0.0 SHM01580
 DO 115 DAY =1,DPY SHM01590
 115 EPAET =EPAET +DPET(DAY) SHM01600
 IF(EPCM(6) .NE. 1.0) EPAET =0.7*EPAET SHM01610

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GO TO 117 SHM01620
PRINT 103, EPAET,MNRD SHM01630
116 READ 103,EPAET,MNRD SHM01640
EMAET =EPAET*(365.0 + MNRD)/404.0 SHM01650
CALL EVPDAY(DPET,EMAET) SHM01660
C READ DAILY FLOW DATA SHM01670
117 DRSF(366)= 0.0 SHM01680
118 DAY =274 SHM01690
READ 1000, (DUM(J),J=1,DPY) SHM01700
PRINT 1000,(DUM(J), J=1,DPY) SHM01710
ISEQ=1 SHM01720
119 DRSF(DAY)=DUM(ISEQ) SHM01730
ISEQ=ISEQ+1 SHM01740
1000 FORMAT(13F6.1) SHM01750
CALL DAYNXT(DAY,DPY) SHM01760
IF(DAY .NE. (396-DPY)) GO TO 119 SHM01770
IF(DIV .EQ. 0.0) GO TO 122 SHM01780
DO 121 DAY = 1,DPY SHM01790
IF(DRSF(DAY) .GT. DIV) GO TO 120 SHM01800
DRSF(DAY)= 0.0 SHM01810
GO TO 121 SHM01820
120 DRSF(DAY)= DRSF(DAY) -DIV SHM01830
121 CONTINUE SHM01840
122 WRITE(6,2) (TITLE(KTA), KTA =1,20) SHM01850
2 FORMAT(1H1,25X,20A4) SHM01860
C WRITE DAILY FLOWS SHM01870
CALL DAYSUM(DRSF,MEDCY,DPY,RATFV,TMRTF) SHM01880
WRITE(6,3) SHM01890
3 FORMAT(1H0,42X,' RECORDED FLOWS ') SHM01900
CALL DAYOUT(DRSF,MEDWY,DPY) SHM01910
WRITE(6,4) (TMRTF(KWD), KWD =1,12),RATFV SHM01920
4 FORMAT(6X,'TOTAL ',2X,12F8.1,2X,F10.1,2X,3HSFD) SHM01930
C READ STORM HYDROGRAPH DATA SHM01940
READ 510,NRHP,NHPT SHM01950
PRINT 510, NRHP,NHPT SHM01960
510 FORMAT(I5,F10.5) SHM01970
IF(NRHP .EQ.0) GO TO 124 SHM01980
DO 123 KRD=1, NRHP SHM01990
READ 90, RHPD(KRD),RHPH(KRD),RHPF(KRD) SHM02000
90 FORMAT(2I5,F10.5) SHM02010
123 WRITE(6,5) KRD,NHPT,RHPD(KRD),RHPH(KRD),RHPF(KRD) SHM02020
5 FORMAT(//5X,'RECORDED HYDROGRAPH',I3/10X,'HYDROGRAPH INTERVAL =', SHM02030
1F5.2,1X,5HHOURS/10X,'CALENDAR DAY OF PEAK =',I5,5X,'HOUR OF DAY=' SHM02040
2,I4,5X,'PEAK FLOW =',F8.1,1X,3HCFS) SHM02050
C INITIALIZE PRECIPITION DATA ARRAYS SHM02060
124 DO 125 DAY = 1,366 SHM02070
DRGPM(DAY)=RGPMB SHM02080
DSSF(DAY)=0.0 SHM02090
DRSGP(DAY) =0.0 SHM02100
DO 125 HOUR = 1,24 SHM02110
125 DRHP(DAY,HOUR) =0.0 SHM02120
C READ AUXILIARY RAIN GAGE DAILY TOTALS SHM02130
READ 500,NSGRD SHM02140
IF(NSGRD .EQ. 0) GO TO 127 SHM02150

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READ 510, SGRT, WSG SHM02160
 IF (CONOPT(3).EQ.1) READ 90, SGRT2, SGMD, WSG2 SHM02170
 DO 126 KRD = 1, NSGRD SHM02180
 READ 103, DRSGP (ISGRD) SHM02190
 READ 500, ISGRD SHM02200
 126 DRSGP (ISGRD) = 0.03937*DRSGP (ISGRD) SHM02210
 C READ RECORDING RAIN GAGE HOURLY TOTALS SHM02220
 127 READ 500, IWBG, YEAR, MONTH, DATE, CN SHM02230
 C PUNCH NO NUMBERS AFTER CN ON YEAR .EQ. 98 CARD SHM02240
 IF (YEAR .GE. 98) GO TO 130 SHM02250
 HRF = 12*(CN-1)+1 SHM02260
 HRL = 12*(CN-1) + 12 SHM02270
 DAY = MEDCY(MCNTH) + DATE + 122 SHM02280
 IF (DAY .GT. DPY) DAY = DAY-DPY SHM02290
 DO 2000 IJK = 1, 12 SHM02300
 IF (DAY .GT. MEDCY(IJK)) GO TO 2000 SHM02310
 MONTH = IJK - 1 SHM02320
 GO TO 2050 SHM02330
 2000 CONTINUE SHM02340
 2050 CONTINUE SHM02350
 READ 1003, (DRHP (DAY, HOUR), HOUR=HRF, HRL) SHM02360
 DO 1291 HOUR= HRF, HRL SHM02370
 1291 DRHP (DAY, HOUR) = 0.03937*DRHP (DAY, HOUR) SHM02380
 1003 FORMAT(12F5.2) SHM02390
 IF (DPY .NE. 366 .OR. MONTH .NE. 2 .OR. DATE .NE. 29) GO TO 127 SHM02400
 DO 129 HOUR=HRF, HRL SHM02410
 DRHP(366, HOUR) = DRHP(60, HOUR) SHM02420
 129 DRHP(60, HOUR) = 0.0 SHM02430
 GO TO 127 SHM02440
 C CALCULATE PRECIPITATION WEIGHTING FACTORS SHM02450
 130 IF (NSGRD .EQ. 0) GO TO 137 SHM02460
 PDAY = 274 SHM02470
 RDPT = 0.0 SHM02480
 DAY = 274 SHM02490
 131 EHSGD = SGRT SHM02500
 IF (SGRT.EQ. 0) EHSGD = 24 SHM02510
 EHSGDF = EHSGD SHM02520
 132 CONTINUE SHM02530
 DO 136 HOUR = 1, 24 SHM02540
 RDPT = RDPT + DRHP (DAY, HOUR) SHM02550
 IF (HOUR .NE. EHSGD) GO TO 136 SHM02560
 IF (RDPT .LE. 0.0) GO TO 133 SHM02570
 IF (SGRT .EQ. 0) PDAY = DAY SHM02580
 DRGPM (PDAY) = (DRSGP (DAY)*WSG + RDPT*(1.0-WSG))/RDPT SHM02590
 IF (CONOPT(1) .NE. 0) DPET(PDAY) = 0.5*DPET(PDAY) SHM02600
 IF (SGRT .NE. 0) PDAY = DAY SHM02610
 RDPT = 0.0 SHM02620
 GO TO 136 SHM02630
 133 IF (DRSGP(DAY) .LE. 0.0) GO TO 135 SHM02640
 DO 134 KHOUR = 1, EHSGD SHM02650
 134 DRHP(DAY, KHOUR) = (WSG*DRSGP(DAY))/EHSGDF SHM02660
 135 IF (SGRT .NE. 0) PDAY=DAY SHM02670
 136 CONTINUE SHM02680
 CALL DAYNXT(DAY, DPY) SHM02690

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IF(DAY .EQ. 274) GO TO 137 SHM02700
IF(CONOPT(3) .EQ.0) GO TO 132 SHM02710
IF(DAY .NE. SGMD) GO TO 132 SHM02720
WSG =WSG2 SHM02730
SGRT = SGRT2 SHM02740
GO TO 131 SHM02750
C ADJUST RAINFALL ANOMALIES SHM02760
137 MXTRH =2*NBT RI SHM02770
IF(CONOPT(2) .EQ. 0) MXTRH = (2*NBT RI -1)/4 +1 SHM02780
NATRH =MXTRH/2 SHM02790
IF(NFTR .GE. 2) GO TO 138 SHM02800
IF(NATRH .LT. 12) CALL PRECHK(DRGPM,DRHP,DRSF,VWIN,SGRT,NATRH) SHM02810
C SET INITIAL VALUES OF VARIABLE PARAMETERS TO BE OPTIMIZED SHM02820
LZC =12.0 SHM02830
BMR = 1.2 SHM02840
SUZC = 1.3 SHM02850
ETLF = 0.25 SHM02860
BUZC =1.50 SHM02870
SIAC =0.90 SHM02880
BIVF =0.90 SHM02890
138 IF(NFTR .EQ.3) GO TO 139 SHM02900
SRX =0.98 SHM02910
NCTRI =NBTRI SHM02920
CSR X =SRX SHM02930
FSRX =SRX SHM02940
CALL RECESS(DRSF,DPY,BFRC,IFRC,AREA,RSBD,RSBIF,NRS,RSBBF) SHM02950
IF(IFRC.GE.0.3)GO TO 139 SHM02960
WRITE(6,6) IFRC SHM02970
6 FORMAT(/10X,'REJECTED IFRC =',F8.4) SHM02980
IFRC =0.1 SHM02990
BIVF =0.0 SHM03000
139 IF(NFTR.GE.2)READ 103,LZC,BMR,SUZC,ETLF,BUZC,SIAC,BIVF,LZS SHM03010
IF(NFTR.EQ.3)READ 540,CSR X,FSRX,NCTRI,CHCAP,IFRC,BFRC SHM03020
540 FORMAT(2F10.5,I5,3F10.5) SHM03030
140 BFHRC =BFRC** (1.0/24.0) SHM03040
BFRL =-ALOG(BFHRC) SHM03050
CALL FIXTRI(CTRI,BTRI,NBT RI,NCTRI) SHM03060
TRIP =NFTR SHM03070
SRX =CSR X SHM03080
KHYD =1 SHM03090
LSHP = .FALSE. SHM03100
DO 141 KIA = 1,5 SHM03110
KPSH(KIA) =0 SHM03120
141 HBF(KIA) =0.0 SHM03130
C POINT OF RETURN FOR NEW TRIP SHM03140
142 IF(KRC .LE. 5) FTX =1.0 SHM03150
IF(DPY .EQ. 366) MEDWY(5) =366 SHM03160
PPH =1.0 SHM03170
IF(. NOT. LRC) PPH =3.0 SHM03180
IF(TRIP .NE. 1) PPH= 4.0 SHM03190
IPPH =PPH SHM03200
FHPP =1.0/PPH SHM03210
IFPRC =IFRC** (FHPP/24.0) SHM03220
IFRL =-ALOG(IFPRC) SHM03230

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VINTCR =FHPP*VINTMR SHM03240
NCTRH =NCTRI SHM03250
IF(CONOPT(2) .EQ. 0) NCTRH =(NCTRI -1)/4 +1 SHM03260
C DETERMINE STORM HOURS FOR ADJUSTING HYDROGRAPH SHAPE VARIABLES SHM03270
IF(NRHP .NE. 0 .AND. TRIP .EQ.2) CALL STRHRS(RHPD,RPHH,IDX, SHM03280
1 IDYE,IHRB,IHRE,NHPT,MXTRH,DPY,NRHP,IBTPR) SHM03290
HSE =0.0 SHM03300
NRTRI =0 SHM03310
PEAI =0.0 SHM03320
SPIF =0.0 SHM03330
OFUS=0.0 SHM03340
OFUSIS =0.0 SHM03350
RHFO=0.0 SHM03360
URHF =0.0 SHM03370
AMIF =0.0 SHM03380
AMNET =0.0 SHM03390
AMPET =0.0 SHM03400
AMPREC =0.0 SHM03410
AMBFI =0.0 SHM03420
AMSE =0.0 SHM03430
KRS =1 SHM03440
KDRS =400 SHM03450
UZS =0.0 SHM03460
IFS =0.0 SHM03470
IF(NFTR .GE. 2) GO TO 145 SHM03480
IF(KRC .NE. 1) GO TO 143 SHM03490
BYLZS =6.00 SHM03500
LZS =BYLZS SHM03510
GO TO 145 SHM03520
143 IF(EMLZS(11) .LT. LZS) LZS =EMLZS(11) SHM03530
LZS =LZS*LZC/PLZC SHM03540
IF(LLZC) LZS = LZC-(LZC-LZS)*(SATFV/RATFV ) SHM03550
IF(ABS(FTX -1.0) .LT. 0.02) GO TO 144 SHM03560
LZS= FTX*BBYLZS*LZC/BLZC SHM03570
IF(LRC .AND.(LZC-LZS).LT. 2.0) LZC = LZS + 2.0 SHM03580
144 IF(TRIP .EQ. 3 .OR. KFFC .EQ. 1) LZS = BBYLZS SHM03590
KFFC=0 SHM03600
145 OCT1BF = 0.05*TMRTF(1) SHM03610
IF(DRSF(274) .LT. 0.05*TMRTF(1))OCT1BF = DRSF(274) SHM03620
IF(DRSF(276) .LT. OCT1BF*BFRC**2) OCT1BF = DRSF(276)/BFRC**2 SHM03630
BYGWS = OCT1BF/(WCFS*BFRL*SQRT(BFRC)) SHM03640
GWS =BYGWS SHM03650
BYLZS =LZS SHM03660
BFNX =GWS*BFRL SHM03670
TFCFS =BFNX*WCFS SHM03680
WRITE(6,7) TRIP,LZC,BMIR,SUZC,ETLF,BUZC,SIAC,BIVF,BFRC,IFRC, SHM03690
1 CSRX,FSRX,NCTRI,CHCAP SHM03700
7 FORMAT(1H1,3X,'TRIAL RUN NUMBER',I3/5X,'PARAMETER VALUES'/10X, SHM03710
1 5HLZC =,3X,F8.4,2X,6HBMIR =,2X,F8.4,2X,6HSUZC =,2X,F8.4,2X, SHM03720
2 6HETLF =,2X,F8.4,2X,6HBUZC =,2X,F8.4,2X,6HSIAC =,2X,F8.4/10X, SHM03730
3 6HBIVF =,2X,F8.4,2X,6HBFRC =,2X,F8.4,2X,6HIFRC =,2X,F8.4,2X, SHM03740
4 6HCSRX =,2X,F8.4,2X,6HFSRX =,2X,F8.4,2X,7HNCTRI =,1X,I8/10X, SHM03750
5 7HCHCAP =,1X,F8.0) SHM03760
WRITE(6,8) LZS,GWS SHM03770

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8 FORMAT(5X,'INITIAL MOISTURE STORAGES, LZS =',F9.4,5X,'GWS =', SHM03780
1 F9.4) SHM03790
  AETX =24.0*EPAET/365.0 SHM03800
  AEX96 =1.2*AETX SHM03810
  AEX90 =0.3*AETX SHM03820
  SIAM =1.2**SIAC SHM03830
UZC=SUZC*AEX90+BUZC*EXP(-2.7*LZS/LZC) SHM03840
IF(UZC.LT.0.25)UZC=0.25 SHM03850
  MONTH =1 SHM03860
  MDAY = 273. SHM03870
  IF(TRIP.EQ. 1) GO TO 147 SHM03880
146 WRITE(6,9) (TITLE(KTA), KTA=1,20) SHM03890
  9 FORMAT(25X,20A4) SHM03900
    WRITE(6,10) YR1,YR2 SHM03910
10 FORMAT(03X,61HOPTIMIZATION OF MODEL INPUT PARAMETERS BASED ON WATE SHM03920
  1R YEAR 19,I2,1H=,I2) SHM03930
    WRITE(6,11) SHM03940
11 FORMAT(5H JUNE) SHM03950
C BEGIN DAY LOOP SHM03960
147 DAY =274 SHM03970
148 CONTINUE SHM03980
  IF(TRIP.NE. 1) GO TO 149 SHM03990
  KDRS = KDRS +1 SHM04000
  IF(RSBD(KRS) .NE. DAY) GO TO 149 SHM04010
  KDRS =1 SHM04020
  KRS =KRS +1 SHM04030
149 CONTINUE SHM04040
  ADIF =0.0 SHM04050
  ADBF =0.0 SHM04060
  TDSF =0.0 SHM04070
  PET =DPET(DAY) SHM04080
  IF(CONDOPT(1) .NE. 2) PET= PET*EPCM(MONTH) SHM04090
  PETU =PET SHM04100
  TFMAX =0.0 SHM04110
  DO 190 HOUR =1,24 SHM04120
  IF(TRIP .NE. 2) GO TO 152 SHM04130
C LOGICAL VARIABLE 'LSHP' SET TRUE DURING DURATION OF RECORDED HYDRO- SHM04140
C GRAPH SO SYNTHESIZED DATA MAY BE SAVED DURING CORRESPONDING PERIOD SHM04150
  IF(KHYD .GT. NRHP) GO TO 152 SHM04160
  IF(IDYB(KHYD) .EQ. DAY .AND. IHRB(KHYD) .EQ. HOUR) LSHP = .TRUE. SHM04170
  IF(KHYD .GE. NRHP) GO TO 150 SHM04180
  IF(IDYB(KHYD+1).EQ. DAY .AND. IHRB(KHYD+1) .EQ.HOUR) KHYD= SHM04190
    1 KHYD+1 SHM04200
150 IF(IDYE(KHYD) .NE. DAY .OR.IHRE(KHYD) .NE. HOUR) GO TO 151 SHM04210
  KHYD = KHYD + 1 SHM04220
  LSHP = .FALSE. SHM04230
151 IF( .NOT. LSHP) GO TO 152 SHM04240
  KPSH(KHYD)= KPSH(KHYD) +1 SHM04250
  IF(KPSH(KHYD) .LT. 171) GO TO 152 SHM04260
    WRITE(6,12) SHM04270
12 FORMAT(5X,'FLCOD HYDROGRAPH ARRAY EXCEEDED, SHORTEN NHPT OR SHIFT SHM04280
  1 TO HOURLY ROUTING') SHM04290
    GO TO 228 SHM04300
152 CONTINUE SHM04310

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1 IF((NSGRD .EQ.0) .AND. (DRHP(DAY,HOUR) .NE. 0.0) .AND.(PET .EQ. SHM04320
1 PETU) .AND. (CONOPT(1) .NE. 0)) PET = 0.5*PET SHM04330
153 IF(HOUR .EQ. SGRT+1) RGPM =DRGPM(DAY) SHM04340
1 IF(HOUR .EQ. 9) HSE =(FWTR*PET)/12.0 SHM04350
1 IF(HOUR .EQ. 21) HSE =0.0 SHM04360
1 PRH =RGPM*DRHP(DAY,HOUR) SHM04370
1 AMPREC =AMPREC + PRH SHM04380
1 ARHF =0.0 SHM04390
C 15 MIN ACCOUNTING AND ROUTING LOOP (60 MINUTES USED FOR ROUGH SHM04400
C ADJUSTMENT, AND 20 MINUTES FOR FINE ADJUSTMENT IN TRIP 1) SHM04410
C DO 182 PRD= 1,IPPH, SHM04420
1 IF(LSHP .AND.CONOPT(2) .EQ. 0 .AND. PRD .NE. 1) KPSH(KHYD) = SHM04430
1 KPSH(KHYD) +1 SHM04440
1 PEBI =0.0 SHM04450
1 PPI=0.0 SHM04460
1 OFR =0.0 SHM04470
1 OFRIS =0.0 SHM04480
1 WI =0.0 SHM04490
1 WEIFS =0.0 SHM04500
1 PEP =FHPP*PRH SHM04510
1 IF(TRIP .GE. 2 .AND.LNPR) CALL PREPRD(RGPM,DRHP,DAY,HOUR,DPY,PRD, SHM04520
1 PEP,PRH) SHM04530
1 IF(PEP .GT. 0.0) GO TO 155 SHM04540
1 IF(OFUS .GT. 0.0) GO TO 157 SHM04550
1 IF(IFS .GT. 0.0) GO TO 167 SHM04560
1 IF(TRIP .EQ. 1) GO TO 181 SHM04570
1 IF(NRTRI .GT. 0) GO TO 169 SHM04580
1 TRHF = 0.0 SHM04590
1 IF(. NOT. LSHP) GO TO 154 SHM04600
1 KHPT =KPSH(KHYD) SHM04610
1 SSR(KHYD,KHPT) =0.0 SHM04620
154 CONTINUE SHM04630
1 IF(RHFD .GT. 0.0) GO TO 178 SHM04640
1 GO TO 181 SHM04650
C RAINFALL UPPER ZONE INTERACTION SHM04660
155 IF(PEP .GE.VINTCR) GO TO 156 SHM04670
1 UZS =UZS +PEP*TPLR SHM04680
1 VINTCR = VINTCR-PEP. SHM04690
1 PPI=0.0 SHM04700
1 PEBI=0.0 SHM04710
1 IF(OFUS .GT.0.0) GO TO 157 SHM04720
1 GO TO 167 SHM04730
156 PPI =PEP-VINTCR SHM04740
1 UZS =UZS+VINTCR*TPLR SHM04750
1 VINTCR =0.0 SHM04760
1 LZSR =LZS/LZC SHM04770
1 UZC =SUZC*AEX90 +BUZC*EXP(-2.7*LZSR) SHM04780
1 IF(UZC .LT. 0.25) UZC =0.25 SHM04790
1 UZRX= 2.0*ABS(UZS/UZC-1.0) +1.0 SHM04800
1 FMR =(1.0/(1.0 +UZRX))*UZRX SHM04810
1 IF(UZS .GT. UZC) FMR =1.0-FMR SHM04820
1 PEBI=PPI*FMR SHM04830
1 UZS =UZS+PPI-PEBI SHM04840
C LOWER ZONE AND GROUNDWATER INFILTRATION SHM04850

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157 LZSR =LZS/LZC SHM04860
EID =4.0*LZSR SHM04870
IF(LZSR .LE.1.0) GO TO 158 SHM04880
EID =4.0+ 2.0*(LZSR=1.0) SHM04890
IF(LZSR .LE.2.0) GO TO 158 SHM04900
EID= 6.0 SHM04910
CMIR =FHPP*SIAM*BMIR/(2.0**EID) SHM04920
158 PEBI =PEBI +OFUS SHM04930
CIVM =BIVF*2.0**LZSR SHM04940
IF(CIVM .LT. 1.0)CIVM =1.0 SHM04950
PEAI =PEBI*PEBI/(2.0*CMIR*CIVM) SHM04960
WI = PEBI*PEBI/(2.0*CMIR) SHM04970
IF(PEBI .GE. CMIR) WI= PEBI-0.5*CMIR SHM04980
IF(PEBI .GE. CMIR*CIVM) PEAI =PEBI -0.5*CMIR*CIVM SHM04990
WEIFS =WI-PEAI SHM05000
IF((PEAI-OFUS) .GT.0.0) GO TO 159 SHM05010
EQD =(OFUS +PEAI)/2.0 SHM05020
GO TO 160 SHM05030
159 EQD = EQDF*((PEAI-OFUS)**0.6) SHM05040
160 IF((OFUS+PEAI) .GT. (2.0*EQD)) EQD =0.5* (OFUS + PEAI) SHM05050
IF((OFUS+PEAI) .LE. 0.001) GO TO 161 SHM05060
OFR=FHPP*OFRF*((OFUS +PEAI)*0.5)**1.67)*((1.0 +.6*((OFUS +
1. PEAI)/(2.0*EQD))**3.0)**1.67) SHM05070
IF(OFR .GT. (0.75*PEAI)) OFR =0.75*PEAI SHM05080
161 IF(FIMP .EQ. 0.0) GO TO 165 SHM05100
162 PEIS =PPI +OFUSIS SHM05110
IF((PEIS-OFUSIS) .GT.0.0) GO TO 163 SHM05120
EQDIS =(OFUSIS + PEIS)/2.0 SHM05130
GO TO 164 SHM05140
163 EQDIS=EQDFIS*((PEIS -OFUSIS)**0.6) SHM05150
164 IF((OFUSIS+PEIS) .GT. (2.0*EQDIS)) EQDIS =0.5*(OFUSIS +PEIS) SHM05160
IF((OFUSIS +PEIS) .LE.0.01) GO TO 165 SHM05170
OFRIS =FHPP*OFRFIS*(( OFUSIS +PEIS)*0.5)**1.67)*((1.0 +0.6*(((
1. OFUSIS +PEIS)/(2.0*EQDFIS))**3.0)**1.67) SHM05180
IF(OFRIS .GT. PEIS) OFRIS =PEIS SHM05190
165 OFUSIS =PEIS-CFRIS SHM05210
OFUS =PEAI -OFR SHM05220
IF(OFUS.GE.0.001)GO TO 166 SHM05230
LZS =LZS +OFUS SHM05240
OFUS=0.0 SHM05250
OFRIS =OFRIS +OFUSIS SHM05260
OFUSIS=0.0 SHM05270
166 LZRX =1.5*ABS(LZS/LZC -1.0) +1.0 SHM05280
FMR =(1.0/(1.0 +LZRX))**LZRX SHM05290
IF(LZS .LT. LZC) FMR = 1.0- FMR*(LZS/LZC) SHM05300
PLZS=FMR*(PEBI-WI) SHM05310
PGW=(1.0-FMR) *(PEBI -WI)*( 1.0 -SUBWF)*FPER SHM05320
GWS=GWS+PGW SHM05330
LZS =LZS +PLZS SHM05340
IFS=IFS+WEIFS*FPER SHM05350
167 SPIF =IFRL*IFS SHM05360
AMIF =AMIF+SPIF SHM05370
ADIF=ADIF+SPIF SHM05380
IFS =IFS-SPIF SHM05390

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IF(IFS.GE.0.0001) GO TO 168 SHM05400
LZS=LZS+IFS SHM05410
IFS=0.0 SHM05420
168 UHFA(1) =FPER*OFR +PPI*FWTR +FIMP*OFRIS +SPIF SHM05430
IF(TRIP .NE. 1) GO TO 169 SHM05440
ARHF =ARHF +UHFA(1) SHM05450
GO TO 181 SHM05460
C ROUTING SHM05470
169 IF(CONOFT (2) .NE. 1) GO TO 170 SHM05480
URHF =URHF +0.25*UHFA(1) SHM05490
IF(PRD.NE. 4) GO TO 178 SHM05500
UHFA(1)=URHF SHM05510
C SAVES SYNTHESIZED DIRECT RUNOFF AND INTER FLOW ENTERING STREAM DURING SHM05520
C DURATION OF RECORDED HYDROGRAPH SHM05530
170 IF( .NOT. LSHP) GO TO 171 SHM05540
KHPT =KPSH(KHYD) SHM05550
IF(CONOFT (2) .EQ.1) SSR(KHYD,KHPT) =4.0*URHF*WCFS SHM05560
IF(CONOFT (2) .EQ. 0) SSR(KHYD,KHPT) =4.0*UHFA(1)*WCFS SHM05570
171 CONTINUE SHM05580
TRHF =0.0 SHM05590
KTRI =NCTRI SHM05600
172 URHF =UHFA(KTRI) SHM05610
IF(URHF .LE. 0.0) GO TO 174 SHM05620
173 TRHF =TRHF +URHF*CTRI(KTRI) SHM05630
UHFA(KTRI + 1) = URHF SHM05640
GO TO 175 SHM05650
174 UHFA(KTRI +1) =0.0 SHM05660
175 KTRI =KTRI -1 SHM05670
IF(KTRI .GE. 1) GO TO 172 SHM05680
176 IF(URHF .LE. 0.0) GO TO 177 SHM05690
NRTRI =NCTRI SHM05700
177 NRTRI =NRTRI -1 SHM05710
UHFA(1) =0.0 SHM05720
URHF =0.0 SHM05730
178 IF(TRIP .LE.2) GO TO 179 SHM05740
IF(TFCFS .LE. 0.5*CHCAP) SRX = CSRX SHM05750
IF((TFCFS .GT. 0.5*CHCAP) .AND. (TFCFS .LT. 2.0*CHCAP)) SRX = CSRX SHM05760
1 +(FSRX -CSRX)*((TFCFS-0.5*CHCAP)/(1.5*CHCAP))**3 SHM05770
IF(TFCFS .GT. 2.0*CHCAP) SRX =FSRX SHM05780
179 RHF1 =TRHF-SRX*(TRHF-RHF0) SHM05790
RHF0 =RHF1 SHM05800
IF(RHF0.LT. RHFMC) RHF0 =0.0 SHM05810
TFCFS =(4.0*RHF1 +CBF-HSE)*WCFS SHM05820
IF(TFCFS .LE. TFMAX) GO TO 180 SHM05830
PRDF =PRD SHM05840
TDFP24 =HOUR SHM05850
IF(PRD .LE.3) TDFP24=( TDFP24-1.0) + 0.15*PRDF SHM05860
TFMAX =TFCFS SHM05870
180 ARHF =ARHF + RHF1 SHM05880
181 IF(VINTCR .LT. FHPP*VINTMR) VINTCR =VINTCR +DPET(DAY)/(24.0/1 FHPP) SHM05890
182 CONTINUE SHM05900
C END OF 15 MINUTE LOOP SHM05910
C ADDING GROUNDERWATER FLOW SHM05920
SHM05930

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183 CBF =GWS*BFRL
  IF(KHYD .GT. NRHP) GO TO 184
  IF(LSHP .AND. (HBF(KHYD) .EQ. 0.0)) HBF(KHYD) = CBF*WCFS
SHM05940
SHM05950
SHM05960
SHM05970
SHM05980
SHM05990
SHM06000
SHM06010
SHM06020
SHM06030
SHM06040
SHM06050
SHM06060
SHM06070
SHM06080
SHM06090
SHM06100
SHM06110
SHM06120
SHM06130
SHM06140
SHM06150
SHM06160
SHM06170
SHM06180
SHM06190
SHM06200
SHM06210
SHM06220
SHM06230
SHM06240
SHM06250
SHM06260
SHM06270
SHM06280
SHM06290
SHM06300
SHM06310
SHM06320
SHM06330
SHM06340
SHM06350
SHM06360
SHM06370
SHM06380
SHM06390
SHM06400
SHM06410
SHM06420
SHM06430
SHM06440
SHM06450
SHM06460
SHM06470

184 GWS =GWS-CBF
  AMBF =AMBF +CBF
  THGR =ARHF +CBF
C  EVAPORATION FROM STREAM SURFACE
185 IF(HSE .GT. THGR) HSE =THGR
  IF(CBF .GT. HSE) ADBF =ADBF +CBF -HSE
  AMSE =AMSE+HSE
  THSF(HOUR) = (THGR -HSE)*WCFS
  IF(TFMAX .LE. 0.0)TFMAX =THSF(HOUR)
  TDSF = TDSF + THSF(HOUR)
SHM06000
SHM06010
SHM06020
SHM06030
SHM06040
SHM06050
SHM06060
SHM06070
SHM06080
SHM06090
SHM06100
SHM06110
SHM06120
SHM06130
SHM06140
SHM06150
SHM06160
SHM06170
SHM06180
SHM06190
SHM06200
SHM06210
SHM06220
SHM06230
SHM06240
SHM06250
SHM06260
SHM06270
SHM06280
SHM06290
SHM06300
SHM06310
SHM06320
SHM06330
SHM06340
SHM06350
SHM06360
SHM06370
SHM06380
SHM06390
SHM06400
SHM06410
SHM06420
SHM06430
SHM06440
SHM06450
SHM06460
SHM06470

C  DRAINING OF UPPER ZONE STORAGE
  UZINFX =(UZS/UZC) -(LZS/LZC)
  IF(UZINFX .LE. 0.0) GO TO 186
  LZSR =LZS/LZC
  UZINLZ =0.003*BMIR*UZC*UZINFX**3.0
  IF(UZINLZ) .GT. UZS) UZINLZ =UZS
  UZS =UZS -UZINLZ
  LZRX =1.5*ABS(LZSR -1.0)+1.0
  FMR =(1.0/(1.0 +LZRX))**LZRX
  IF(LZS .LT. LZC) FMR=1.0-FMR*LZSR
  PGW =(1.0-FMR)*UZINLZ*(1.0 -SUBWF)*FPER
  PLZS=FMR*UZINLZ
  LZS =LZS +PLZS
  GWS =GWS +PGW
SHM06000
SHM06010
SHM06020
SHM06030
SHM06040
SHM06050
SHM06060
SHM06070
SHM06080
SHM06090
SHM06100
SHM06110
SHM06120
SHM06130
SHM06140
SHM06150
SHM06160
SHM06170
SHM06180
SHM06190
SHM06200
SHM06210
SHM06220
SHM06230
SHM06240
SHM06250
SHM06260
SHM06270
SHM06280
SHM06290
SHM06300
SHM06310
SHM06320
SHM06330
SHM06340
SHM06350
SHM06360
SHM06370
SHM06380
SHM06390
SHM06400
SHM06410
SHM06420
SHM06430
SHM06440
SHM06450
SHM06460
SHM06470

C  4 PM ADJUSTMENTS OF VARIUS VALUES
186 IF(HOUR .NE. 16) GO TO 190
  AEX90 =0.9*(AEX90 +PET)
  AEX96 =0.96*(AEX96 +PET)
SHM06210
SHM06220
SHM06230
SHM06240
SHM06250
SHM06260
SHM06270
SHM06280
SHM06290
SHM06300
SHM06310
SHM06320
SHM06330
SHM06340
SHM06350
SHM06360
SHM06370
SHM06380
SHM06390
SHM06400
SHM06410
SHM06420
SHM06430
SHM06440
SHM06450
SHM06460
SHM06470

C  INFILTRATION CORRECTION
  SIAM =(AEX96/AETX)**SIAC
  IF(SIAM .LT. 0.33) SIAM =0.33
  IF(PET .EQ. 0.0) GO TO 190
SHM06210
SHM06220
SHM06230
SHM06240
SHM06250
SHM06260
SHM06270
SHM06280
SHM06290
SHM06300
SHM06310
SHM06320
SHM06330
SHM06340
SHM06350
SHM06360
SHM06370
SHM06380
SHM06390
SHM06400
SHM06410
SHM06420
SHM06430
SHM06440
SHM06450
SHM06460
SHM06470

C  EVAP-TRANS LOSS FROM GROUND WATER
  GWET =GWS*GWETF*PET*FPER
  GWS =GWS-GWET
  AMPET =AMPET +PET
  IF(PET .GE. UZS) GO TO 187
  UZS =UZS -PET
  AMNET =AMNET +PET
  GO TO 190
SHM06210
SHM06220
SHM06230
SHM06240
SHM06250
SHM06260
SHM06270
SHM06280
SHM06290
SHM06300
SHM06310
SHM06320
SHM06330
SHM06340
SHM06350
SHM06360
SHM06370
SHM06380
SHM06390
SHM06400
SHM06410
SHM06420
SHM06430
SHM06440
SHM06450
SHM06460
SHM06470

187 PET = PET-UZS
  AMNET =AMNET + UZS
  UZS =0.0
  LZSR = LZS/LZC
  IF(PET .GE. ETLF*LZSR) GO TO 188
  SET = PET*(1.0- PET/(2.0*ETLF*LZSR))
  GO TO 189
SHM06210
SHM06220
SHM06230
SHM06240
SHM06250
SHM06260
SHM06270
SHM06280
SHM06290
SHM06300
SHM06310
SHM06320
SHM06330
SHM06340
SHM06350
SHM06360
SHM06370
SHM06380
SHM06390
SHM06400
SHM06410
SHM06420
SHM06430
SHM06440
SHM06450
SHM06460
SHM06470

188 SET =0 .5*ETLF*LZSR
189 LZS =LZS-SET
  AMNET =AMNET +SET
190 CONTINUE
SHM06210
SHM06220
SHM06230
SHM06240
SHM06250
SHM06260
SHM06270
SHM06280
SHM06290
SHM06300
SHM06310
SHM06320
SHM06330
SHM06340
SHM06350
SHM06360
SHM06370
SHM06380
SHM06390
SHM06400
SHM06410
SHM06420
SHM06430
SHM06440
SHM06450
SHM06460
SHM06470

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>; END OF HOUR LOOP
DSSF(DAY) =TDSF/24.0 SHM06480
IF(TRIP.EQ. 1) GO TO 192 SHM06490
IF(TFMAX .LE. RMPF) GO TO 192 SHM06500
IF(DAY .EQ. 366) MDAY =337 SHM06510
DATE =MOD(DAY,MDAY) SHM06520
WRITE(6,13) DATE, (THSF(HOUR),HOUR=1,12) SHM06530
13 FORMAT(1H/,1X/,1X,I4,2X,2HAM,1X,6F8.1,3X,6F8.1) SHM06540
WRITE(6,14) (THSF(HOUR),HOUR=13,24) , DSSF(DAY) SHM06550
14 FORMAT(1H/,6X,2HMPM,1X,6F8.1,3X,7F8.1) SHM06560
IF(TDFP24 .LT. 12.0) GO TO 191 SHM06570
TDFP12 =TDFP24 -12.0 SHM06580
WRITE(6,15) TFMAX, TDFP12 SHM06590
15 FORMAT(1H/,10X,'MAXIMUM=',F8.1,2X,'C.F.S.',5X,'TIME',3X,F5.2,2X,
1 4HP.M.) SHM06600
GO TO 192 SHM06610
191 WRITE(6,16) TFMAX,TDFP24 SHM06620
16 FORMAT(1H/,10X,8HMAXIMUM=,F8.1,2X,6HC.F.S.,5X,4HTIME,3X,F5.2,2X,
1 4HA.M.) SHM06630
192 CONTINUE SHM06640
IF(TRIP .EQ. 1 .AND. .NOT. LRC .AND. KDRS .LE. 3 .AND. IFRC .GT.
1 0.1) SIFRS(KDRS,KRS=1) = ADIF*VWIN SHM06650
IF(TRIP .EQ. 1 .AND. KDRS .LE. 3) SBFRS(KDRS,KRS=1) =ADBF*VWIN SHM06660
C MONTHLY SUMMARY STORAGE SHM06670
IF(DAY .NE. MEDWY(MONTH)) GO TO 206 SHM06680
TMPREC(MONTH) = AMPREC SHM06690
AMPREC =0.0 SHM06700
TMBF(MONTH) = AMBF SHM06710
AMBFR =0.0 SHM06720
TMIF(MONTH) = AMIF SHM06730
AMIF =0.0 SHM06740
TMSE(MONTH) = AMSE SHM06750
AMSE =0.0 SHM06760
TMPET(MONTH) = AMPET SHM06770
AMPET =0.0 SHM06780
TMNET(MONTH) = AMNET SHM06790
AMNET =0.0 SHM06800
EMGWS(MONTH) = GWS SHM06810
UZC=SUZC*AEX90 +BUZC*EXP(-2.7*LZS/LZC) SHM06820
IF(UZC .LT. 0.25) UZC =0.25 SHM06830
EMUZC(MONTH)=UZC SHM06840
EMUZS(MONTH) =UZS SHM06850
EMSIAM(MONTH) =SIAM SHM06860
EMLZS(MONTH) =LZS SHM06870
EMIFS(MONTH) =IFS SHM06880
IF(MONTH .EQ. 5) MEDWY(5) =59 SHM06890
MDAY =MEDWY(MONTH) SHM06900
IF(TRIP .EQ. 1) GO TO 205 SHM06910
193 GO TO (194,195,196,197,198,199,200,201,202,203,204,
1 205),MONTH SHM06920
194 WRITE(6,17) SHM06930
17 FORMAT(1H/,4HJULY) SHM06940
GO TO 205 SHM06950
195 WRITE(6,18) SHM06960
SHM06970
SHM06980
SHM06990
SHM07000
SHM07010

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18 FORMAT(1H/,6HAUGUST) SHM07020
  GO TO 205 SHM07030
196 WRITE(6,19) SHM07040
  19 FORMAT(1H/,9HSEPTEMBER) SHM07050
  GO TO 205 SHM07060
197 WRITE(6,20) SHM07070
  20 FORMAT(1H/,7HOCTOBER) SHM07080
  GO TO 205 SHM07090
198 WRITE(6,21) SHM07100
  21 FORMAT(1H/,8HNOVEMBER) SHM07110
  GO TO 205 SHM07120
199 WRITE(6,22) SHM07130
  22 FORMAT(1H/,8HDECEMBER) SHM07140
  GO TO 205 SHM07150
200 WRITE(6,23) SHM07160
  23 FORMAT(1H/,7HJANUARY) SHM07170
  GO TO 205 SHM07180
201 WRITE(6,24) SHM07190
  24 FORMAT(1H/,8HFEBRUARY) SHM07200
  GO TO 205 SHM07210
202 WRITE(6,25) SHM07220
  25 FORMAT(1H/,5HMARCH) SHM07230
  GO TO 205 SHM07240
203 WRITE(6,26) SHM07250
  26 FORMAT(1H/,5HAPRIL) SHM07260
  GO TO 205 SHM07270
204 WRITE(6,27) SHM07280
  27 FORMAT(1H/,3HMAY) SHM07290
205 MONTH =MONTH +1 SHM07300
C END OF DAY LOOP SHM07310
206 CALL DAYNXT(DAY,DPY) SHM07320
  IF(DAY .NE. 274) GO TO 148 SHM07330
  IF(TRIP .NE. 2) GO TO 208 SHM07340
C ADJUST BASE FLOW FOR AVERAGE VALUE DURING STORM SHM07350
  IF(NRHP .EQ. 0) GO TO 208 SHM07360
  DO 207 KHYD =1,NRHP SHM07370
    DAY =IDYB(KHYD) SHM07380
    IF(DSSF(DAY) .GT. HBF(KHYD)) GO TO 207 SHM07390
    HBF(KHYD) =(HBF(KHYD) +DSSF(DAY))/2.0 SHM07400
207 CONTINUE SHM07410
208 IF(TRIP .NE. 1) WRITE(6,28) (TITLE(KTA), KTA=1,20,1) SHM07420
28 FORMAT(1H1,25X,20A4) SHM07430
C ANNUAL SUMMARY SHM07440
  APREC = 0.0 SHM07450
  ABFV=0.0 SHM07460
  ASEV=0.0 SHM07470
  ANET=0.0 SHM07480
  APET=0.0 SHM07490
  AIFV = 0.0 SHM07500
  DO 209 MONTH =1,12 SHM07510
    APREC =APREC +TMPREC(MONTH) SHM07520
    ABFV=ABFV +TMBF(MONTH) SHM07530
    ASEV=ASEV+ TMSE(MONTH) SHM07540
    ANET=ANET +TMNET(MONTH) SHM07550

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APET=APET + TMPET(MONTH) SHM07560
209 AIFV=AIFV + TMIF(MONTH) SHM07570
  WRITE(6,29) SHM07580
29  FORMAT(1H//44X, 23H SYNTHESIZED FLOWS) SHM07590
210 IF(TRIP .EQ. 1) WRITE(6,30) SHM07600
30  FORMAT(//5X, 'SUMMARY WHILE OPTIMIZING VOLUME VARIABLES') SHM07610
211 CALL DAYSUM(DSSF, MEDCY, DPY, SATFV, TMSTF) SHM07620
  IF(TRIP .EQ. 1) GO TO 212 SHM07630
  CALL DAYOUT(DSSF, MEDWY, DPY) SHM07640
212 WRITE(6,31) (TMSTF(KWD), KWD=1,12), SATFV SHM07650
31  FORMAT(1X, 9H SYNTHETIC, 3X, 12F8.1, 2X, F10.1, 2X, 3HSFD) SHM07660
  DO 213 MONTH = 1,12 SHM07670
213 TMSTFI(MONTH) = TMSTF(MONTH)/VWIN SHM07680
  SATFVI = SATFV/VWIN SHM07690
  WRITE(6,32) (TMSTFI(KWD), KWD=1,12), SATFVI SHM07700
32  FORMAT(1X, 5HTOTAL, 8X, 12F8.3, 4X, F7.3, 2X, 6HINCHES) SHM07710
  DO 214 MONTH = 1,12 SHM07720
  TMDF(MONTH) = TMSTFI(MONTH)-TMIF(MONTH)-TMBF(MONTH) + SHM07730
1  TMSE(MONTH) SHM07740
214 IF(TMDF(MONTH) .LT. 0.0) TMDF(MONTH)=0.0 SHM07750
  AOFV=SATFVI-AIFV-ABFV+ASEV SHM07760
  IF(AOFV .LT. 0.0) AOFV=0.0 SHM07770
  WRITE(6,33) (TMDF(KWD), KWD=1,12), AOFV SHM07780
33  FORMAT(1X, 'OVERLAND', 5X, 12F8.3, 4X, F7.3, 2X, 'INCHES') SHM07790
  WRITE(6,34) (TMIF(KWD), KWD=1,12), AIFV SHM07800
34  FORMAT(1X, 9HINTERFLOW, 4X, 12F8.3, 4X, F7.3, 2X, 6HINCHES) SHM07810
  WRITE(6,35) (TMBF(KWD), KWD=1,12), ABFV SHM07820
35  FORMAT(1X, 4HBASE, 9X, 12F8.3, 4X, F7.3, 2X, 6HINCHES) SHM07830
  WRITE(6,36) (TMSE(KWD), KWD=1,12), ASEV SHM07840
36  FORMAT(1X, 9HSTRM EVAP, 4X, 12F8.3, 4X, F7.3, 2X, 6HINCHES) SHM07850
  WRITE(6,37) (TMPREC(KWD), KWD=1,12), APREC SHM07860
37  FORMAT(1X, 6HPRECIP, 7X, 12F8.2, 3X, F8.2, 2X, 6HINCHES) SHM07870
  WRITE(6,38) (TMNET(KWD), KWD=1,12), ANET SHM07880
38  FORMAT(1X, 'EV.P/TRAN-NET', 2X, 12F8.3, 3X, F7.3, 2X, 'INCHES') SHM07890
  WRITE(6,39) (TMPET(KWD), KWD=1,12), APET SHM07900
39  FORMAT(3X, 10H-POTENTIAL, 2X, 12F8.3, 3X, F7.3, 2X, 6HINCHES) SHM07910
  WRITE(6,40) (EMUZS(KWD), KWD=1,12) SHM07920
40  FORMAT(1X, 12H STORAGES-UZS, 2X, 12F8.3, 12X, 6HINCHES) SHM07930
  WRITE(6,41) (EMLZS(KWD), KWD=1,12) SHM07940
41  FORMAT(10X, 3HLZS, 2X, 12F8.3, 12X, 6HINCHES) SHM07950
  WRITE(6,42) (EMIFS(KWD), KWD=1,12) SHM07960
42  FORMAT(10X, 3HIFS, 2X, 12F8.3, 12X, 6HINCHES) SHM07970
  WRITE(6,43) (EMGWS(KWD), KWD=1,12) SHM07980
43  FORMAT(10X, 3HGWS, 2X, 12F8.3, 12X, 6HINCHES) SHM07990
  WRITE(6,44) (EMUZC(KWD), KWD=1,12) SHM08000
44  FORMAT(1X, 12H INDICES-UZC, 2X, 12F8.3) SHM08010
  WRITE(6,45) (EMSIAM(KWD), KWD=1,12) SHM08020
45  FORMAT(9X, 4HSIAM, 2X, 12F8.3) SHM08030
  AMBER =(LZS-BYLZS)*FPER+(UZS + IFS + GWS - BYGWS)*(1.0 - FWTR SHM08040
1  ) + SATFVI +ANET*FPER+ASEV+APREC SHM08050
  WRITE(6,46) AMBER SHM08060
46  FORMAT(1H/, 7HBALANCE, 5X, F10.4, 2X, 6HINCHES) SHM08070
  C ESTABLISH WHETHER MONTH IS PREDOMINATELY BASE FLOW OR DIRECT RUNOFF SHM08080
  NDFM=0 SHM08090

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MONTH1=1 SHM08100
IF(FTX .LT. 0.95) MONTH1=4 SHM08110
DO 216 MONTH=1,12 SHM08120
XMPFT(MONTH) =0.0 SHM08130
IF(MONTH .LT. MONTH1) GO TO 216 SHM08140
  IF(TMSTFI(MONTH) .GT. 0.001) GO TO 215 SHM08150
XMPFT(MONTH)=1.0 SHM08160
GO TO 216 SHM08170
215 IF(TMBF(MONTH)/TMSTFI(MONTH) .GT. 0.5) XMPFT(MONTH) =1.0 SHM08180
  IF(TMOF(MONTH)/TMSTFI(MONTH) .LT. 0.5) GO TO 216 SHM08190
  NOFM =NOFM+1 SHM08200
  XMPFT(MONTH) =2.0 SHM08210
216 CONTINUE SHM08220
C NATURE OF TRIPS SHM08230
C TRIP 1 OPTIMIZE VOLUME VARIABLES WHILE BYPASSING ROUTING SHM08240
C TRIP 2 SET FLOOD HYDROGRAPH VARIABLES. CSRX,FSRX,NCTRI,CHCAP SHM08250
C TRIP 3 FINAL RUN WITH OPTIMIZED VALUES SHM08260
217 IF(TRIP .EQ. 1) GO TO 218 SHM08270
  KRC =MNRC +1 SHM08280
  IF(TRIP .EQ. 2) GO TO 226 SHM08290
  GO TO 228 SHM08300
C SYSTEMATIC ADJUSTMENT OF VOLUME VARIABLES CONVERGING ON OPTIMUM VALUES SHM08310
218 KRC=KRC +1 SHM08320
  KBRC =KBRC +1 SHM08330
  PLZC =LZC SHM08340
  PBMIR =BMIR SHM08350
  PSUZC =SUZC SHM08360
  PETLF =ETLF SHM08370
  PBUZC =BUZC SHM08380
  PSIAC =SIAC SHM08390
C ADJUST FIVE VOLUME VARIABLES. LZC,SUZC,ETLF,BUZC,SIAC SHM08400
  CALL SETFVP(LZC,SUZC,ETLF,BUZC,SIAC,TMSTF,TMRTF,TMPREC,TMPET, SHM08410
    1 EMLZS,SSQM,LRC,XMPFT,FTX,NOFM,LBUZC,LETLF,LLZC,APREC,APET) SHM08420
C ADJUST INTERFLOW VOLUME CONSTANT DURING FINE ADJUSTMENT PHASE SHM08430
  FNCTRH =NCTRH SHM08440
  IF(.NOT. LRC .AND. IFRC .GT. 0.1) CALL SETBIV(BIVF,NRS,IFRC,RSBF,SHM08450
    1 SIFRS,FNCTRH) SHM08460
C ADJUST INFILTRATION RATE CONSTANT BMIR SHM08470
  IF(.NOT. LBMIR) GO TO 219 SHM08480
  BMIR =0.9*BMIR SHM08490
  GO TO 220 SHM08500
219 IF(ABS(FTX-1.0) .GT. 0.02 .AND. KRC.GT. 5) IFT =2 SHM08510
  CALL SETBMI(BMIR,NRS,BFRC,RSBBF,SBFRS,FNCTRH,IFT) SHM08520
220 IF((KRC .GT. 6) .AND. (LZC .GT. 29.0)) LLZC= .TRUE. SHM08530
  IF((KRC .GT. 6) .AND. (ETLF .GT. 0.59)) LETLF = .TRUE. SHM08540
  IF((KRC .GT. 6) .AND. (BUZC .GT. 3.9)) LBUZC= .TRUE. SHM08550
  IF(.NOT. LLZC) GOTO 221 SHM08560
  LZC =PLZC*SATFV/RATFV SHM08570
  WRITE(6,47) LZC SHM08580
47 FORMAT(/2X,'LZC WAS CHANGED TO',F6.2,' BASED ON ANNUAL RUNOFF VOLUME') SHM08590
  1ME')
221 IF(KRC .LT. 6 .OR. BMIR .LT. 20.0) GO TO 222 SHM08600
  LBMIR = .TRUE. SHM08610
  BMIR= 20.0 SHM08620

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222 IF(SSQM .LE. SSQM .AND. ((KRC .GE. MNRC .AND. KBRC .GE. 2) .OR. 1 (.NOT. LRC))) GO TO 224 SHM08640
 1 IF(SSQM .LE. SSQM) GO TO 142 SHM08650
 1 IF(KRC .GE. MNRC) KRC=KRC-1 SHM08660
 BLZC=PLZC SHM08670
 BBMIR=PBMIR SHM08680
 BSUZC=PSUZC SHM08690
 BETLF= PETLF SHM08700
 BBUZC=PBUZC SHM08710
 BSIAC =PSIAC SHM08720
 SSSQM=SSQM SHM08730
 BBYLZS=BYLZS SHM08740
 KBRC=0 SHM08750
 IF(SSQM .LT. 0.15 .AND. LRC) GO TO 223 SHM08760
 GO TO 142 SHM08770
 223 LRC=.FALSE. SHM08780
 WRITE(6,48) SHM08790
 48 FORMAT(/5X,'SHIFT TO FINE ADJUSTMENT BEGINNING AT BEST ROUGH ADJUSSHM08810
 1TMENT POINT') SHM08820
 SSSQM= 1000.0 SHM08830
 GO TO 225 SHM08840
 224 CONTINUE SHM08850
 IF(LRC) GO TO 223 SHM08860
 IF(TRIP .GE. NLTR) GO TO 228 SHM08870
 TRIP =TRIP+1 SHM08880
 225 LZC =BLZC SHM08890
 BMIR = BBMIR SHM08900
 SUZC = BSUZC SHM08910
 ETLF = BETLF SHM08920
 BUZC = BBUZC SHM08930
 SIAC = BSIAC SHM08940
 KFFC = 1 SHM08950
 GO TO 142 SHM08960
 226 IF(NRHP .EQ. 0) GO TO 227 SHM08970
 C CORRECT SYNTHESIZED RUNOFF TO RECORDED VOLUMES SHM08980
 CALL ADJHYD(IDYB, IDYE, IHRB, IHRE, KPSH, DPY, HBF, NRHP, DSSF, DRSF, SSR,
 1 LSHA) SHM08990
 C ESTABLISH STORM AND OVERALL OPTIMUM VALUES FOR SRX AND NCTRI SHM09000
 CALL SETHRP(CTR, BTR, WCFS, CONOPT(2), HBF, LSHA, SSR, NHPT, KPSH,
 1 IBTPR, SRX, CSRX, FSRX, CHCAP, NRHP, RHPF, NCTRI, NBTRI) SHM09010
 IF(NCTRI .EQ. 0) GO TO 228 SHM09020
 227 IF(TRIP .GE. NLTR) GO TO 228 SHM09030
 TRIP = TRIP +1 SHM09040
 GO TO 142 SHM09050
 228 CONTINUE SHM09060
 IF(NSYC .LT. NSYT) GO TO 100 SHM09070
 STOP SHM09080
 END SHM09090
 SUBROUTINE ADJHYD(IDYB, IDYE, IHRB, IHRE, KPSH, DPY, HBF, NRHP, DSSF,
 1 DRSF, SSR, LSHA) SHM09100
 C ADJUSTS SYNTHESIZED FLOW VOLUME TO MATCH RECORDED VOLUME FOR SETTING SHM09110
 C HYDROGRAPH ROUTTING PARAMETERS SHM09120
 DIMENSION IDYB(5), IDYE(5), IHRB(5), KPSH(5), SSR(5,170),
 1 DSSF(366), DRSF(366), HBF(5), LSHA(5), IHRE(5) SHM09130
 SHM09140 SHM09150 SHM09160 SHM09170

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LOGICAL LSHA,LSHP SHM09180
INTEGER DAY,DPY SHM09190
LSHP = .FALSE. SHM09200
KRHP = 1 SHM09210
DAY = 274 SHM09220
100 CONTINUE SHM09230
IF(LSHP) GO TO 102 SHM09240
IF(IDYB(KRHP) .NE. DAY) GO TO 107 SHM09250
101 HTH= IHRB(KRHP) SHM09260
HBFM= 1.0 SHM09270
IF(DSSF(DAY) .LT. HBF(KRHP)) HBFM= 0.0 SHM09280
TSHV= (24.0-HTH)*(DSSF(DAY) -HBF(KRHP))*HBFM/24.0 SHM09290
HBFM = 1.0 SHM09300
IF(DRSF(DAY) .LT. HBF(KRHP)) HBFM= 0.0 SHM09310
TRHV = (24.0 -HTH)*(DRSF(DAY) -HBF(KRHP))*HBFM/24.0 SHM09320
IF(IDYE(KRHP) .EQ. DAY) GO TO 104 SHM09330
LSHP = .TRUE. SHM09340
GO TO 107 SHM09350
102 IF(DSSF(DAY) .LT. HBF(KRHP)) GO TO 103 SHM09360
TSHV =TSHV + DSSF(DAY) -HBF(KRHP) SHM09370
103 IF(DRSF(DAY) .LT. HBF(KRHP)) GO TO 104 SHM09380
TRHV = TRHV + DRSE(DAY) -HBF(KRHP) SHM09390
104 CONTINUE SHM09400
IF(IDYE(KRHP) .NE. DAY) GO TO 107 SHM09410
HTH = IHRE(KRHP) SHM09420
TSHV =TSHV -(24.0-HTH)*(DSSF(DAY) -HBF(KRHP))/24.0 SHM09430
TRHV = TRHV-(24.0 -HTH)*(DRSE(DAY) -HBF(KRHP))/24.0 SHM09440
LSHP =.FALSE. SHM09450
SHM= TRHV/TSHV SHM09460
LSHA(KRHP) = .TRUE. SHM09470
IF(SHM .GT. 8.0 .OR. SHM .LT. 0.125), LSHA(KRHP) = .FALSE. SHM09480
IF( .NOT. LSHA(KRHP)) GO TO 106 SHM09490
KPCH =KPSH(KRHP) SHM09500
DO 105 KHPT = 1,KPCH SHM09510
SSR(KRHP,KHPT) = SHM*SSR(KRHP,KHPT) SHM09520
106 WRITE(6,1) KRHP,SHM SHM09530
1 FORMAT(//10X,'VOLUME ADJUSTMENT FACTOR FOR HYDROGRAPH',I2, SHM09540
1 'EQUALS',F10.4) SHM09550
105 KRHP = KRHP + 1 SHM09560
IF(KRHP .GT. NRHP) RETURN SHM09570
IF(IDYB(KRHP) .EQ. IDYE(KRHP-1)) GO TO 101 SHM09580
107 CALL DAYNXT(DAY,DPY) SHM09590
IF(DAY .NE. 274) GO TO 100 SHM09600
RETURN SHM09610
END SHM09620
SUBROUTINE DAYSUM(DRSF,MEDCY,DPY,ATFV,TMTFWY) SHM09630
C SUMS DAILY VALUES TO GET MONTHLY AND ANNUAL TOTALS SHM09640
DIMENSION DRSE(366),EMATF(13),MEDCY(12),TMTFCY(12),TMTFWY(12) SHM09650
C INTEGER DAY,DPY SHM09660
C SUM ANNUAL AND CUMULATIVE MONTHLY FLOWS SHM09670
EMATF(1) = 0.0 SHM09680
ATF = 0.0 SHM09690
DO 101 DAY = 1,365 SHM09700
ATF =ATF +DRSE(DAY) SHM09710

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DO 100 KMO = 2,12 SHM09720
.00 IF(DAY .EQ. MEDCY(KMO)) EMATF(KMO) = ATF SHM09730
.01 CONTINUE SHM09740
EMATF(13) =ATF SHM09750
ATFV =ATF +DRSF(366) SHM09760
CALCULATE MONTHLY FLOWS SHM09770
DO 102 KMO = 1,12 SHM09780
102 TMTFCY(KMO) =EMATF(KMO + 1) -EMATF(KMO) SHM09790
TMTFCY(2) = TMTFCY(2) + DRSF(366) SHM09800
CONVERT MONTHLY FLOWS TO AWATER YEAR ORDER SHM09810
DO 103 KMO =1,9 SHM09820
103 TMTFWY(KMO+3) =TMTFCY(KMO) SHM09830
DO 104 KMO = 10,12 SHM09840
104 TMTFWY(KMO-9) = TMTFCY(KMO) SHM09850
RETURN SHM09860
END SHM09870
SUBROUTINE FIXTRI(CTRI,BTRI,NBTRI,NCTRI) SHM09880
FIX VALUES OF THE TIME ROUTING INCREMENTS TO MATCH REQUIRED TOTAL SHM09890
NUMBER OF VALUES SHM09900
DIMENSION AWSBIT(99),BTRI(99),CTRI(99) SHM09910
IF(NCTRI .GT. 99) GO TO 101 SHM09920
IF(NBTRI .NE. NCTRI) GO TO 102 SHM09930
DO 100 KRD = 1,99 SHM09940
100 CTRI(KRD) = BTRI(KRD) SHM09950
RETURN SHM09960
101 WRITE(6,1) NCTRI SHM09970
1 FORMAT(5X,'NCTRI OF',I5,1X,'EXCEEDS MAXIMUM VALUE OF 99, 99 USED') SHM09980
NCTRI = 99 SHM09990
102 DO 103 KIA = 1,99 SHM10000
103 AWSBIT(KIA) = 0.0 SHM10010
FNTRI = NCTRI SHM10020
KB1 = 0 SHM10030
KB2 = 1 SHM10040
KB3 = 0 SHM10050
104 KB1 =KB1 + 1 SHM10060
IF(KB1 .GT. NCTRI) GO TO 107 SHM10070
KB4 = 0 SHM10080
WSBIT = BTRI(KB1)/FNTRI SHM10090
105 KB4 = KB4 + 1 SHM10100
IF(KB4 .GT. NCTRI) GOTO 104 SHM10110
AWSBIT(KB2) =AWSBIT(KB2) + WSBIT SHM10120
KB3 = KB3 +1 SHM10130
IF(KB3 .LT. NCTRI) GO TO 106 SHM10140
KB3 = 0 SHM10150
KB2 = KB2 + 1 SHM10160
106 GO TO 105 SHM10170
107 DO 108 KB5 = 1,99 SHM10180
108 CTRI (KB5) = AWSBIT(KB5) SHM10190
RETURN SHM10200
END SHM10210
SUBROUTINE PRECHK(DRGPM,DRHP,DRSF,VWIN,SGRT,NATRH) SHM10220
C CHECKS PRECIPITATION-STREAMFLOW ANOMALIES AND ADJUSTS PRECIPITATION SHM10230
C WHERE NECESSARY SHM10240
DIMENSION DRGPM(366),DRSF(366),DRHP(366,24) SHM10250

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INTEGER DAY,HCUR,SGRT SHM10260
AHP = 0.0 SHM10270
NRHA = 24-NATRH SHM10280
RGPM = DRGPM(90) SHM10290
DAY=90 SHM10300
RMWR=1.25 SHM10310
DAY=DAY+1 SHM10320
DO IF(DAY .GT. 200 .OR. VWIN.GT.750.0) RMWR=2.00 SHM10330
RFRISE =(DRSF(DAY) -DRSF(DAY-1))/VWIN SHM10340
DO 101 HOUR = 1,24 SHM10350
IF(HOUR .EQ. SGRT+1) RGPM =DRGPM(DAY) SHM10360
AHP =AHP + DRHP(DAY,HOUR)*RGPM SHM10370
IF(HOUR .NE. NRHA) GO TO 101 SHM10380
RWRAIN =AHP SHM10390
AHP = 0.0 SHM10400
101 CONTINUE SHM10410
IF(RFRISE .GT. RWRAIN .AND.RFRISE .GT. 0.1) GO TO 102 SHM10420
IF((RWRAIN .GT. RMWR .AND. RFRISE .LT. 0.02*RWRAIN) .OR. (RWRAIN 1 .GT. 3.00 .AND. RFRISE .LT. 0.05*RWRAIN)) GO TO 104 SHM10430
GO TO 108 SHM10440
102 IF(RWRAIN .GT. 0.05) GO TO 103 SHM10450
RAA = RFRISE*2.0-RWRAIN +1.0 SHM10460
DRHP(DAY,13) = RAA SHM10470
WRITE(6,1) DAY, RAA SHM10480
1 FORMAT(/10X,'FOR DAY',I4,1X,'RAIN ADDED OF',F7.2) SHM10490
GO TO 108 SHM10500
103 RAM =2.0*RFRISE/RWRAIN SHM10510
GO TO 105 SHM10520
104 RAM = 10.0 *RFRISE/RWRAIN SHM10530
105 IF(RAM .LT. 0.0) GO TO 108 SHM10540
WRITE(6,2) DAY, RAM, RWRAIN SHM10550
2 FORMAT(/5X,'FCR DAY',I4,1X,'RAIN ADJUSTMENT MULTIPLIER IS',F8.4, 1 1X,'RECORDED RAIN IS',F7.2) SHM10560
DO 106 HOUR = 1,NRHA SHM10570
106 DRHP(DAY,HOUR) =DRHP(DAY,HOUR)*RAM SHM10580
IF(NATRH .EQ. 0) GO TO 108 SHM10590
NFRHA =NRHA +1 SHM10600
DO 107 HOUR =NFRHA,24 SHM10610
107 DRHP(DAY-1,HOUR) =DRHP(DAY-1,HOUR)*RAM SHM10620
108 IF(DAY .NE. 273) GO TO 100 SHM10630
RETURN SHM10640
END SHM10650
SUBROUTINE RECESS(DRSF,DPY,BFRC,IFRC,AREA,RSBD,RSBIF,NRS,RSBBF) SHM10660
ESTABLISHES RECESSION SEQUENCES SHM10670
DIMENSION DRSF(366),LBFO(20),NDRS(20),RSBBF(20),RSBD(20), 1 RSBFRC(20),RSBIF(20),RSIFRC(20),RSTF(50,20) SHM10680
LOGICAL LBFO SHM10690
INTEGER DAY,DPY,RSBD,RSL SHM10700
REAL IFRC SHM10710
MRSL = 9 SHM10720
BFRC = 0.9 SHM10730
IFRC = 0.05 SHM10740
FRERS = 0.1*SQRT(AREA) SHM10750
100 DO 101 KSD = 1,50 SHM10760
101

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DO 101 KRS = 1,20
101 RSTF(KSD,KRS) = 0.0
  KRS = 0
  DAY = 274
  BEGIN NEW SEQUENCE
102 IF(KRS .GE. 20) GO TO 109
  KRS = KRS + 1
  KSD = 1
  RSF1 = DRSF(DAY)
  CALL DAYNXT(DAY,DPY)
  IF(DAY .EQ. 274) GO TO 107
  RSF2 = DRSF(DAY)
  RSBD(KRS) = DAY
  IF(RSF2 .LT. (RSF1+FRERS).AND. (RSF2 .GT. 0.4*AREA .OR. RSF2 .GT.
  1 10.0)) GO TO 103
  KRS = KRS - 1
  GO TO 102
103 RSTF(1,KRS) = RSF2
  RSFM = RSF2
104 KSD = KSD + 1
  CALL DAYNXT(DAY,DPY)
  IF(DAY .EQ. 274) GO TO 107
  RSFN = DRSF(DAY)
  IF(RSFN .LT. (RSFM +FRERS) .AND. RSFN .GT. 0.0) GO TO 106
  IF(KSD .GE. MRSL) GO TO 102
  NDRS(KRS) = 0
  DO 105 KSD = 1,MRSL
105 RSTF(KSD,KRS) = 0.0
  KRS = KRS - 1
  GO TO 102
106 IF(RSFN .LT. RSFM) RSFM = RSFN
  RSTF (KSD,KRS) = RSFN
  NDRS(KRS) = KSD
  IF(KSD .GE. 50) GO TO 102
  GO TO 104
107 IF(KSD .GE. MRSL) GO TO 109
  NTRS = KRS - 1
  DO 108 KSD = 1,MRSL
108 RSTF(KSD,KRS) = 0.0
  GO TO 110
109 NTRS = KRS
110 CONTINUE
  IF(NTRS .GE. 3) GO TO 111
  IF(MRSL .LT. 7) RETURN
  MRSL = 6
  GO TO 100
C  WRITE OUT ESTABLISHED ARRAY OF FLOW SEQUENCES
111 WRITE(6,1)
1  FORMAT(5X,'FLOW SEQUENCES USED TO ESTIMATE RECEDITION CONSTANTS')
  DO 113 KRS = 1, NTRS
  NDRSC = NDRS(KRS)
  DO 112 KSD = 2,NDRSC
112 RSTF(KSD-1,KRS) = RSTF(KSD,KRS)
  NDRS(KRS) = NDRS(KRS) - 1
  SHM10800
  SHM10810
  SHM10820
  SHM10830
  SHM10840
  SHM10850
  SHM10860
  SHM10870
  SHM10880
  SHM10890
  SHM10900
  SHM10910
  SHM10920
  SHM10930
  SHM10940
  SHM10950
  SHM10960
  SHM10970
  SHM10980
  SHM10990
  SHM11000
  SHM11010
  SHM11020
  SHM11030
  SHM11040
  SHM11050
  SHM11060
  SHM11070
  SHM11080
  SHM11090
  SHM11100
  SHM11110
  SHM11120
  SHM11130
  SHM11140
  SHM11150
  SHM11160
  SHM11170
  SHM11180
  SHM11190
  SHM11200
  SHM11210
  SHM11220
  SHM11230
  SHM11240
  SHM11250
  SHM11260
  SHM11270
  SHM11280
  SHM11290
  SHM11300
  SHM11310
  SHM11320
  SHM11330

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NDRSC = NDRSC - 1 SHM11340
WRITE(6,2) KRS,(RSTF(KSD,KRS),KSD=1,NDRSC) SHM11350
2 FORMAT(/10X,I2,5(10F8.1/12X)) SHM11360
13 CONTINUE SHM11370
DETERMINE RECESSION CONSTANS FROM EACH SEQUENCE SHM11380
114 DO 116 KRS = 1,NTRS SHM11390
  IF((RSTF(1,KRS) .LT. 0.4*AREA) .AND. (RSTF(2,KRS) .GT. 0.8* SHM11400
  1 (RSTF(1,KRS))) GO TO 115 SHM11410
  LBFO(KRS) = .FALSE. SHM11420
  CALL SET2RC(RSTF,KRS,NDRS(KRS),RSIFRC(KRS),RSBFRC(KRS),LBFO(KRS)) SHM11430
  IF(LBFO(KRS) .OR. RSBFRC(KRS) .GT. 1.2 .OR. RSBFRC(KRS) .LT. 0.6 SHM11440
  1 .OR. RSIFRC(KRS) .GT. 0.8 .OR. RSIFRC(KRS) .LT. (-0.4))GO TO 115 SHM11450
  GO TO 116 SHM11460
15 LBFO(KRS) = .TRUE. SHM11470
  CALL SET1RC(RSTF,KRS,NDRS(KRS),RSBFRC(KRS)) SHM11480
16 CONTINUE SHM11490
CALCULATE WEIGHTED AVERAGE RECESSION CONSTANTS SHM11500
  BFRC = 0.0 SHM11510
  IFRC = 0.0 SHM11520
  ABFSL = 0.0 SHM11530
  AIFSL = 0.0 SHM11540
  DO 118 KRS = 1,NTRS SHM11550
  IF(RSBFRC(KRS) .GT. 1.2 .OR. RSBFRC(KRS) .LT. 0.6) GO TO 117 SHM11560
  RSL = NDRS(KRS) SHM11570
  BFRC = BFRC + RSBFRC(KRS)*RSL SHM11580
  ABFSL = ABFSL + RSL SHM11590
  IF(LBFO(KRS)) GO TO 118 SHM11600
  IF(RSL .GE. 20.0) RSL = 20.0 SHM11610
  IFRC = IFRC + RSIFRC(KRS)*RSL SHM11620
  AIFSL = AIFSL + RSL SHM11630
  GO TO 118 SHM11640
17 WRITE(6,3) KRS SHM11650
3  FORMAT(10X,'SEQUENCE',I3,1X,' OMITTED IN AVERAGING') SHM11660
18 CONTINUE SHM11670
  WRITE(6,4) ABFSL,AIFSL SHM11680
4  FORMAT(10X,'BASE FLOW DAYS =',F5.0,2X,'INTERFLOW DAYS =',F5.0) SHM11690
  BFRC = BFRC/ABFSL SHM11700
  IFRC = IFRC/AIFSL SHM11710
  IF(BFRC .GT. 0.99) BFRC = 0.99 SHM11720
  IF(BFRC .LT. 0.70) BFRC = 0.70 SHM11730
  KSQ = 0 SHM11740
  DO 119 KRS = 1,NTRS SHM11750
  IF(LBFO(KRS)) GO TO 119 SHM11760
  CALL SETRBF(RSTF,NDRS,KRS,BFRC,IFRC,CRSBIF,CRSBBF) SHM11770
  IF(CRSBIF .GT. 95000.0 .OR. CRSBBF .LT. 0.0) GO TO 119 SHM11780
  IF(CRSBIF .LT. 0.0) CRSBIF = 0.0 SHM11790
  KSQ = KSQ + 1 SHM11800
  RSBD(KSQ) = RSBD(KRS) SHM11810
  RSBI(FSQ) = CRSBIF SHM11820
  RSBBF(KSQ) = CRSBBF SHM11830
119 CONTINUE SHM11840
  NRS = KSQ SHM11850
  DO 120 KSQ = 1,NRS SHM11860
  DAY = RSBD(KSQ) SHM11870

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120 CALL DAYNXT(DAY,DPY) SHM11880
      RSBD(KSQ) = DAY SHM11890
      DO 121 KSQ = 1,NRS SHM11900
      CRSBTF = RSBIF(KSQ) + RSBBF(KSQ) SHM11910
121 WRITE(6,5) KSQ,RSBD(KSQ),RSBIF(KSQ),RSBBF(KSQ),CRSBTF SHM11920
      5 FORMAT(10X,'REVISED FLOW SEQUENCE',I3,1X,'BEGINS ON DAY',I4, SHM11930
      1 1X,'AT INTERFLOW =',F7.2,1X,'CFS,  BASE FLOW =',F7.2,1X,'CFS, SHM11940
      2  TOTAL =',F7.2,1X,'CFS') SHM11950
      RETURN SHM11960
      END SHM11970
      SUBROUTINE SETBIV(BIVF,NRS,IFRC,RSBIF,SIFRS,FNCTRH) SHM11980
      SETS BEST VALUE OF BASIC INTERFLOW VOLUME FACTOR SHM11990
      DIMENSION RSBIF(20),SIFRS(3,20) SHM12000
      REAL IFRC SHM12010
      ARSTR = 0.0 SHM12020
      DO 101 KRS = 1,NRS SHM12030
      RIF = RSBIF(KRS)/IFRC SHM12040
      DO 100 KDY = 1,3 SHM12050
      RIF = RIF*IFRC SHM12060
      SIF = SIFRS(KDY,KRS)/IFRC**(FNCTRH/48.0) SHM12070
      RSTR = 0.0 SHM12080
      IF(RIF .GT. 0.0) RSTR = SIF/RIF SHM12090
      IF(RSTR .GT. 3.0 .OR. (SIF .GT. 0.0 .AND. RIF .EQ. 0.0))RSTR=3.0 SHM12100
      ARSTR = ARSTR +RSTR SHM12110
      WRITE(6,1) KRS,KDY,SIF,RIF SHM12120
      1 FORMAT(10X,'KRS=',I3,2X,'KDY=',I2,2X,'SIF=',F7.1,5X,'RIF=', SHM12130
      1  F7.1) SHM12140
100  CONTINUE SHM12150
101  CONTINUE SHM12160
      TIRD = NRS*3 SHM12170
      PBIVF = BIVF SHM12180
      BIVF = 0.40 SHM12190
      IF(ARSTR .GT. 0.0) BIVF = ((PBIVF-0.40)*TIRD)/ARSTR + 0.40 SHM12200
      WRITE(6,2) PBIVF,BIVF SHM12210
      2 FORMAT(5X,'BIVF CHANGED FROM',F6.2,2X,'TO',F6.2//) SHM12220
      RETURN SHM12230
      END SHM12240
      SUBROUTINE SETBMI(BMIR,NRS,BFRC,RSBBF,SBFRS,FNCTRH,IFT) SHM12250
      C SETS BEST VALUE OF BASIC MAXIMUM INFILTRATION RATE WITHIN WATERSHED SHM12260
      DIMENSION RSBBF(20),SBFRS(3,20) SHM12270
      ARSTR = 0.0 SHM12280
      DO 101 KRS = IFT,NRS SHM12290
      RBF = RSBBF(KRS)/BFRC SHM12300
      DO 100 KDY = 1,3 SHM12310
      RBF = RBF*BFRC SHM12320
      SBF = SBFRS(KDY,KRS)/BFRC**(FNCTRH/48.0) SHM12330
      RSTR = SBF/RBF SHM12340
      IF(RSTR .GT. 3.0) RSTR =3.0 SHM12350
      ARSTR = ARSTR + RSTR SHM12360
      WRITE(6,1) KRS,KDY,SBF,RBF SHM12370
      1 FORMAT(10X,'KRS=',I3,2X,'KDY=',I2,2X,'SBF=',F7.1,5X,'RBF=', SHM12380
      1  F7.1) SHM12390
100  CONTINUE SHM12400
101  CONTINUE SHM12410

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TBRD = (NRS + 1-IFT)*3 SHM12420
ARSTR = ARSTR/TBRD SHM12430
ARSTR = ARSTR**1.3 SHM12440
PBMIR = BMIR SHM12450
BMIR = PBMIR/ARSTR SHM12460
WRITE(6,2) PBmir,BMIR SHM12470
2 FORMAT(5X,'BMIR CHANGED FROM',F6.2,2X,'TO',F6.2//) SHM12480
RETURN SHM12490
END SHM12500
SUBROUTINE SETFDI(MFDP,TMSTF,TMRTF,SSQM) SHM12510
SETS VALUES OF FLOW DEVIATION INDICES SHM12520
DIMENSION MFDP(12),TMRTF(12),TMSTF(12) SHM12530
REAL MFDP SHM12540
DO 101 MONTH = 1,12 SHM12550
IF(MONTH .LE. 2) SSQM = 0.0 SHM12560
SMFX = TMSTF(MONTH) + 20.0 SHM12570
RMFX = TMRTF(MONTH) + 20.0 SHM12580
MFDP(MONTH) = SMFX/RMFX - 1.0 SHM12590
IF(MFDP(MONTH) .GT. 8.0) MFDP(MONTH) = 8.0 SHM12600
IF(MFDP(MONTH) .LT. 0.0) MFDP(MONTH) = 1.0 -RMFX/SMFX SHM12610
IF(MFDP(MONTH) .LT. (-8.0)) MFDP(MONTH) = -8.0 SHM12620
100 SSQM = SSQM + MFDP(MONTH)*MFDP(MONTH) SHM12630
101 CONTINUE SHM12640
WRITE(6,1) (MFDP(MONTH), MONTH=1,12), SSQM SHM12650
1 FORMAT(//2X,'MONTHLY DEVIATIONS',/16X,12(F7.3,1X), 'SSQM =',F7.3) SHM12660
RETURN SHM12670
END SHM12680
SUBROUTINE SETFVP(LZC,SUZC,ETLF,BUZC,SIAC,TMSTF,TMRTF,TMPREC, SHM12690
1 TMPET,EMLZS,SSQM,LRC,XMPFT,FTX,NOFM,LBUZC,LETLF,LLZC,APREC,APET) SHM12700
C SETS BEST VALUES OF FLOW VOLUME PARAMETERS SHM12710
DIMENSION EMLZS(12),MFDP(12),MXA(12),TMPET(12),TMPREC(12), SHM12720
1 TMRTF(12),TMSTF(12),XMPFT(12) SHM12730
LOGICAL LBUZC,LETLF,LLZC,LRC SHM12740
REAL LZC,MFDP SHM12750
CALL SETFDI(MFDP,TMSTF,TMRTF,SSQM) SHM12760
IF((MFDP(2)+MFDP(3)) .GT. 2.0 .AND. FTX .LT. 1.05) FTX= 0.9 SHM12770
IF((MFDP(2)+MFDP(3)) .LT. (-2.0).AND. FTX .GT. 0.95) FTX=1.1 SHM12780
C ADJUSTMENT OF LZC BASED ON MONTHS WHERE OVER HALF OF TOTAL SHM12790
C SYNTHESIZED RUNOFF IS OVERLAND FLOW, MINIMUM OF TWO MONTHS SHM12800
C WITH GREATEST RUNOFF USEC SHM12810
PLZC = LZC SHM12820
FNOFM = NOFM SHM12830
IF(NOFM .GT. 2) GO TO 103 SHM12840
M1R = 2 SHM12850
M2R = 1 SHM12860
IF(TMRTF(2).GT. TMRTF(1)) GO TO 100 SHM12870
M1R = 1 SHM12880
M2R = 2 SHM12890
100 DO 102 MONTH = 3,12 SHM12900
IF(TMRTF(MONTH) .LT. TMRTF(M2R)) GO TO 102 SHM12910
IF(TMRTF(MONTH) .GT. TMRTF(M1R)) GO TO 101 SHM12920
M2R = MONTH SHM12930
GO TO 102 SHM12940
101 M2R = M1R SHM12950

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112  PRM2 = TMPREC(MONTH) SHM13500
      CONTINUE SHM13510
      FSUZC = MFDP(M1SP) + MFDP(M2SP) SHM13520
      IF(ABS(XMPFT(12) - 1.0) .GT. 0.2) GO TO 113 SHM13530
      FSUZC = FSUZC + MFDP(12) SHM13540
      M12 = 12 SHM13550
113  IF(ABS(XMPFT(11) - 1.0) .GT. 0.2) GO TO 114 SHM13560
      FSUZC = FSUZC + MFDP(11) SHM13570
      M11 = 11 SHM13580
114  IF(FSUZC .GT. 1.0) FSUZC = 1.0 SHM13590
      IF(FSUZC .LT. (-1.0)) FSUZC = -1.0 SHM13600
      IF(FSUZC .GT. 0.0) SUZC = (FSUZC + 1.0)*SUZC SHM13610
      IF(FSUZC .LE. 0.0) SUZC = SUZC/(1.0-FSUZC) SHM13620
      WRITE(6,3) SUZC,M1SP,M2SP,M11,M12 SHM13630
3   FORMAT(4X,'SUZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',4I3) SHM13640
      IF(SUZC .LT. 0.3 .AND. LRC) SUZC = 0.3 SHM13650
      IF(SUZC .GT. 3.0 .AND. LRC) SUZC = 3.0 SHM13660
      ADJUSTMENT OF ETLF BASED ON SUMMER MONTHS OF RAINFALL EXCEEDING TWO SHM13670
      INCHES OR NEED TO PREVENT MOISTURE BUILDUP SHM13680
      IF(EMLZS(12) .LT. PLZC .OR. EMLZS(11) .LT. PLZC .OR. APREC .GT. SHM13690
      1. 1.5*APET) GO TO 115 SHM13700
      FETLF = 1.0 SHM13710
      MXA(1) = 13 SHM13720
      KWSM = 1 SHM13730
      GO TO 120 SHM13740
115  SWSMD = 0.0 SHM13750
      KWSM = 0 SHM13760
      DO 116 MONTH = 1,12 SHM13770
      IF((MONTH .GT. MBWS .OR. MONTH .GT. 2) .AND. (MONTH .LT. MBDS SHM13780
      1 .AND. MONTH .LT. 9)) GO TO 116 SHM13790
      IF(TMPREC(MONTH) .LT. 2.0) GO TO 116 SHM13800
      SWSMD = SWSMD + MFDP(MONTH) SHM13810
      KWSM = KWSM + 1 SHM13820
      MXA(KWSM) = MONTH SHM13830
116  CONTINUE SHM13840
      IF(KWSM .GE. 1) GO TO 117 SHM13850
      MXA(1) = M1R SHM13860
      KWSM = 1 SHM13870
      FETLF = 5.0*MFDP(M1R) SHM13880
      GO TO 120 SHM13890
117  WSM = KWSM SHM13900
      IF(.NOT. LETLF .OR. KWSM .EQ. 1) GO TO 119 SHM13910
      EMFDP = 0.0 SHM13920
      DO 118 MONTH = 1,KWSM SHM13930
      KM1 = MXA(MONTH) SHM13940
      IF(MFDP(KM1) .LT. EMFDP) GO TO 118 SHM13950
      EMFDP = MFDP(KM1) SHM13960
      KM2 = MONTH SHM13970
118  CONTINUE SHM13980
      MXA(KM2) = 0 SHM13990
      SWSMD = SWSMD - EMFDP SHM14000
      WSM = WSM - 1.0 SHM14010
      FETLF = 1.0*2*SWSMD/WSM SHM14020
119  IF(FETLF .GT. 1.0) FETLF = 1.0 SHM14030
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IF(FETLF .LT. -1.0) FETLF = -1.0 SHM14040
IF(FETLF .GT. 0.0) ETLF = (FETLF + 1.0)*ETLF SHM14050
IF(FETLF .LT. 0.0) ETLF = ETLF/(1.0-FETLF) SHM14060
4 WRITE(6,4) ETLF, (MXA(KWD), KWD = 1, KWSM) SHM14070
FORMAT(4X, 'ETLF WAS CHANGED TO', F6.2, ' BASED ON MONTHS', 12I3) SHM14080
IF(ETLF .LT. 0.05 .AND. LRC) ETLF = 0.05 SHM14090
IF(ETLF .GT. 0.6 .AND. LRC) ETLF = 0.6 SHM14100
ADJUSTMENT OF BUZC BASED ON SEPTEMBER, NOVEMBER, AND DECEMBER SHM14110
  KM1 = 12 SHM14120
  KM2 = 2 SHM14130
  KM3 = 3 SHM14140
  FBUZC = 0.4*(MFDP(12) + MFDP(2) + MFDP(3)) SHM14150
  IF(.NOT. LBUZC) GO TO 121 SHM14160
  FBUZC = 0.4*(MFDP(9) + MFDP(10) + MFDP(11)) SHM14170
  KM1 = 9 SHM14180
  KM2 = 10 SHM14190
  KM3 = 11 SHM14200
121 IF(FBUZC .GT. 1.0) FBUZC = 1.0 SHM14210
  IF(FBUZC .LT. -1.0) FBUZC = -1.0 SHM14220
  IF(FBUZC .GT. 0.0) BUZC = (FBUZC + 1.0)*BUZC SHM14230
  IF(FBUZC .LE. 0.0) BUZC = BUZC/(1.0-FBUZC) SHM14240
  WRITE(6,5) BUZC, KM1, KM2, KM3 SHM14250
5 FORMAT(4X, 'BUZC WAS CHANGED TO', F6.2, ' BASED ON MONTHS', 3I3) SHM14260
  IF(BUZC .LT. 0.2 .AND. LRC) BUZC = 0.2 SHM14270
  IF(BUZC .GT. 4.0 .AND. LRC) BUZC = 4.0 SHM14280
ADJUSTMENT OF SIAC BASED ON THREE FIRST MOISTURE EXCESS AND THREE SHM14290
  FIRST MOISTURE DEFICIENT MONTHS SHM14300
  KM1 = MBDS SHM14310
  KM2 = MBDS + 1 SHM14320
  KM3 = MBDS - 1 SHM14330
  KM4 = 0 SHM14340
  KM5 = 0 SHM14350
  KM6 = 0 SHM14360
  WFDX = 0.0 SHM14370
  IF(SIAC .GT. 1.0) GO TO 122 SHM14380
  WFDX = (MFDP(MBWS) + MFDP(MBWS+1) + MFDP(MBWS+2))/3.0 SHM14390
  IF(SIAC .GT. 0.6) WFDX = WFDX*(1.0- SIAC)/0.4 SHM14400
  KM4 = MBWS SHM14410
  KM5 = MBWS + 1 SHM14420
  KM6 = MBWS + 2 SHM14430
122 SFDX = (MFDP(MBDS) + MFDP(MBDS+1) + MFDP(MBDS-1))/3.0 SHM14440
  FSIAC = 1.5*(SFDX-WFDX) SHM14450
  IF(FSIAC .GT. 1.0) FSIAC = 1.0 SHM14460
  IF(FSIAC .LE. -1.0) FSIAC = -1.0 SHM14470
  IF(SIAC .LT. 0.02) SIAC = 0.02 SHM14480
  IF(FSIAC .GT. 0.0) SIAC = (FSIAC + 1.0)*SIAC SHM14490
  IF(FSIAC .LE. 0.0) SIAC = SIAC/(1.0-FSIAC) SHM14500
  WRITE(6,6) SIAC, KM4, KM5, KM6, KM3, KM1, KM2 SHM14510
  FORMAT(4X, 'SIAC WAS CHANGED TO', F6.2, ' BASED ON MONTHS', 6I3) SHM14520
  IF(SIAC .LT. 0.02 .AND. LRC) SIAC = 0.00 SHM14530
  IF(SIAC .GT. 4.0 .AND. LRC) SIAC = 4.0 SHM14540
  RETURN SHM14550
  END SHM14560
  SUBROUTINE SETHR(CTRI, BTRI, WCFS, CONOP2, HBF, LSHA, SSR, NHPT, KPSH, SHM14570

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1 IBTPR,SRX,CSRX,FSRX,CHCAP,NRHP,RHPF,NCTRI,NBTRI) SHM14580
SETS BEST VALUES OF HYDROGRAPH ROUTING PARAMETERS SHM14590
DIMENSION BTRI(99),CTRI(99),HBF(5),HSRX(5),KPSH(5),LSHA(5), SHM14600
1 HNTRI(5),RHPF(5),SRR(5,170),SSR(5,170),TSRX(7) SHM14610
LOGICAL LSHA SHM14620
INTEGER CONOP2,HNTRI,SNTRI SHM14630
REAL NHPT SHM14640
MHTP = 1 SHM14650
IF(CONOP2 .EQ. 0) MHTP = 4 SHM14660
MXTRI = 2*NBTRI SHM14670
MNTRI = NBTRI/2 SHM14680
TSRX(1) = 0.995 SHM14690
TSRX(2) = 0.99 SHM14700
TSRX(3) = 0.985 SHM14710
TSRX(4) = 0.98 SHM14720
TSRX(5) = 0.96 SHM14730
TSRX(6) = 0.93 SHM14740
TSRX(7) = 0.90 SHM14750
LNIBRS = 0 SHM14760
DO 112 KHYD = 1,NRHP SHM14770
IF( .NOT. LSHA(KHYD)) GO TO 112 SHM14780
KPCH = KPSH(KHYD) SHM14790
NCTRI = MNTRI SHM14800
CALL FIXTRI(CTRI,BTRI,NBTRI,NCTRI) SHM14810
KH1 = 1 SHM14820
KH2 = 1 SHM14830
KH3 = 1 SHM14840
SDRSP = 80.0*CHCAP SHM14850
SNTRI = MXTRI SHM14860
100 SRX = TSRX(KH1) SHM14870
IF(KH2 .EQ. 2) LNIBRS = NIBRS SHM14880
WRITE(6,1) NCTRI,SRX SHM14890
1 FORMAT(//15X,'TRIAL VALUE OF NCTRI =',I3,',', SRX =',F6.3) SHM14900
CALL TIMERT( SSR,SRR,CTRI,NCTRI,KHYD,KPCH) SHM14910
CSRX = SRX SHM14920
FSRX = SRX SHM14930
CALL STDRRT(SRR,CSRX,FSRX,CHCAP,CONOP2,IBTPS,SHPF,KHYD,HBF(KHYD), SHM14940
1 NHPT,KPCH,IBTPR) SHM14950
LNTRI = NCTRI SHM14960
NIRTS = IBTPS- IBTPR*MHTP SHM14970
NIBRS = TABS(NIRTS) SHM14980
DRSP = ABS(SHPF - RHPF(KHYD)) SHM14990
IF(NIRTS .EQ. 0 .OR. (KH2 .EQ. 2 .AND. NIBRS .GE. LNIBRS) .OR. SHM15000
1 RHPF(KHYD) .GT. 1.2*SHPF) GO TO 103 SHM15010
IF(NIRTS .GE. 1) GO TO 109 SHM15020
101 NCTRI = NCTRI -NIRTS SHM15030
IF(NCTRI .LT. MNTRI) NCTRI = MNTRI SHM15040
IF(NCTRI .GT. MXTRI) GO TO 106 SHM15050
102 CALL FIXTRI(CTRI,BTRI,NBTRI,NCTRI) SHM15060
KH2 = 2 SHM15070
GO TO 100 SHM15080
103 IF(DRSP .GT. SDRSP) GO TO 108 SHM15090
SNTRI = LNTRI SHM15100
SDRSP = DRSP SHM15110

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104  KH3 = 2 SHM15120
      KH1 = KH1 + 1 SHM15130
      IF(KH1 .EQ. 8) GO TO 105 SHM15140
      KH2 = 1 SHM15150
      GO TO 100 SHM15160
105  HNTRI(KHYD) = LNTRI SHM15170
      HSRX(KHYD) = SRX SHM15180
      GO TO 111 SHM15190
106  IF(KH1 .GE. 2 .AND. KH3 .EQ. 2 .AND. DRSP .GE. SDRSP) GO TO 108 SHM15200
      NCTRI = MXTRI SHM15210
      CALL FIXTRI(CTRI,BTRI,NBTRI,NCTRI) SHM15220
      IF(KH2 .EQ. 2 .AND. KH1 .EQ. 1 .AND. SHPF .GT. RHPF(KHYD)) GO SHM15230
      1 TO 107 SHM15240
      IF( KH2 .EQ. 2 .OR. KH1 .GE. 2) GO TO 109 SHM15250
      GO TO 102 SHM15260
107  HNTRI(KHYD) = MXTRI SHM15270
      HSRX(KHYD) = 0.995 SHM15280
      GO TO 111 SHM15290
108  HSRX(KHYD) = TSRX(KH1-1) SHM15300
      HNTRI(KHYD) = SNTRI SHM15310
      GO TO 111 SHM15320
109  IF(NCTRI .GT. MNTRI .AND. NCTRI .LT. MXTRI) GO TO 101 SHM15330
      IF(DRSP .GT. SDRSP) GO TO 110 SHM15340
      SDRSP = DRSP SHM15350
      SNTRI = LNTRI SHM15360
      GO TO 104 SHM15370
110  HNTRI(KHYD) = NCTRI SHM15380
      HSRX(KHYD) = 0.995 SHM15390
      IF(KH1 .GE. 2) HSRX(KHYD) = TSRX(KH1 - 1) SHM15400
111  IF(HSRX(KHYD) .LT. 0.91 .AND. SHPF .LT. 0.5*RHPF(KHYD)) LSHA(KHYD) SHM15410
      1 = .FALSE. SHM15420
      IF(.NOT. LSHA(KHYD)) GO TO 112 SHM15430
      WRITE(6,2) KHYD,HNTRI(KHYD),HSRX(KHYD) SHM15440
      2 FORMAT(1DX,'FCR STORM ',I2,' NCTRI =',I3,' SRX =',F6.3) SHM15450
112  CONTINUE SHM15460
      KPA = 1 SHM15470
113  ARHPF = 0.0 SHM15480
      APPKP = 0.0 SHM15490
      DO 114 KHYD = 1, NRHP SHM15500
      IF( .NOT. LSHA(KHYD)) GO TO 114 SHM15510
      CHPV = HNTRI(KHYD) SHM15520
      IF(KPA .EQ. 2) CHPV = HSRX(KHYD) SHM15530
      APPKP = APPKP + CHPV*RHPF(KHYD) SHM15540
      ARHPF = ARHPF + RHPF(KHYD) SHM15550
114  CONTINUE SHM15560
      WAPV = APPKP/ARHPF SHM15570
      IF(KPA .EQ. 2) GO TO 115 SHM15580
      NCTRI = WAPV + 0.5 SHM15590
      WRITE(6,3) NCTRI SHM15600
      3 FORMAT(//10X,'OPTIMUM NCTRI =',I3) SHM15610
      IF(NCTRI .EQ. 0) RETURN SHM15620
      CALL FIXTRI(CTRI,BTRI,NBTRI,NCTRI) SHM15630
      WRITE(6,4) (CTRI(KTRI), KTRI = 1,NCTRI) SHM15640
      4 FORMAT(18X,'CTRI ARE'//9(16X,11F8.4/)) SHM15650

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5   WRITE(6,5) SHM15660
      5   FORMAT(18X,'WARNING- THE USER MAY HAVE TO ADJUST THESE VALUES TO SHM15670
      1 MAKE THEM ADD TO ONE TO COMPENSATE FOR ROUNDING.') SHM15680
      KPA = 2 SHM15690
      GO TO 113 SHM15700
115  SRX = WAPV SHM15710
      CSRX = SRX SHM15720
      FSRX = SRX SHM15730
      CALL SETSRP(CONOP2,NRHP,LSHA,RHPF,HSRX,CHCAP,SSR,SRR,CTRI,CSRX, SHM15740
      1 FSRX,KHYD,IBTPS,SHPF,NCTRI,HBF,NHPT,KPSH,IBTPR) SHM15750
      SRX = CSRX SHM15760
      RETURN SHM15770
      END SHM15780
      SUBROUTINE SETRBF(RSTF,NDRS,KRS,BFRC,IFRC,CRSBIF,CRSBBF) SHM15790
      SETS VALUES OF INTERFLOW AND BASE FLOW AT RECESSION BEGINNING SHM15800
      DIMENSION RSTF(50,20),NDRS(20) SHM15810
      REAL*8 RA1,RA2,RA3,RA4,RA5,RA6 SHM15820
      REAL IFRC SHM15830
      RA1 = 0.0 SHM15840
      RA2 = 0.0 SHM15850
      RA3 = 0.0 SHM15860
      RA4 = 0.0 SHM15870
      RA5 = 0.0 SHM15880
      MNDRS = 12 SHM15890
      IF(NDRS(KRS) .LT. 12) MNDRS = NDRS(KRS) SHM15900
      IF(IFRC .GE. 0.3) GO TO 101 SHM15910
      CRSBIF = 0.0 SHM15920
      DO 100 KSD = 1,MNDRS SHM15930
      RA1 = RA1 + BFRC**(2*KSD) SHM15940
100   RA4 = RA4 + RSTF(KSD,KRS)*(BFRC**KSD) SHM15950
      CRS0BF = RA4/RA1 SHM15960
      CRSBBF = CRS0BF*BFRC SHM15970
      RETURN SHM15980
101   CRSBIF = 100000.0 SHM15990
      DO 102 KSD = 1,MNDRS SHM16000
      RA1 = RA1 + BFRC**(2*KSD) SHM16010
      RA2 = RA2 + IFRC**(2*KSD) SHM16020
      RA3 = RA3 + (BFRC*IFRC)**KSD SHM16030
      RA4 = RA4 + RSTF(KSD,KRS)*(BFRC**KSD) SHM16040
      RA5 = RA5 + RSTF(KSD,KRS)*(IFRC**KSD) SHM16050
102   CONTINUE SHM16060
      RA6 = RA1*RA2 - RA3**2 SHM16070
      IF(RA6 .EQ. 0.0) RETURN SHM16080
      CRS0IF = -(RA3/RA6)*RA4 +(RA1/RA6)*RA5 SHM16090
      CRSBIF = CRS0IF*IFRC SHM16100
      CRS0BF = (RA2/RA6)*RA4 -(RA3/RA6)*RA5 SHM16110
      CRSBBF = CRS0BF*BFRC SHM16120
      RETURN SHM16130
      END SHM16140
      SUBROUTINE SETSRP(CONOP2,NRHP,LSHA,RHPF,HSRX,CHCAP,SSR,SRR,CTRI, SHM16150
      1 CSRX,FSRX,KHYD,IBTPS,SHPF,NCTRI,HBF,NHPT,KPSH,IBTPR) SHM16160
      SETS BEST VALUES OF STORAGE ROUTING PARAMETERS SHM16170
      DIMENSION CTRI(99),HBF(5),HSRX(5),KPSH(5),LSHA(5),RHPF(5), SHM16180
      1 SRR(5,170),SSR(5,170) SHM16190

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LOGICAL LSHA
INTEGER CONOP2
REAL NHPT
KLCCA = 1
SRX = CSRX
EPS = 0.000001
NRHP = NRHP
DO 100 KHYD = 1, NORHP
IF( .NOT. LSHA(KHYD)) NRHP = NRHP-1
100 CONTINUE
IF(NRHP .LE. 2) GO TO 103
FIND REGRESSION LINE FOR DETERMINING CSRX, WHEN NRHP EXCEEDS 3
RA1 = 0.0
RA2 = 0.0
RA3 = 0.0
RA4 = 0.0
FNRHP = NRHP
DO 101 KHYD = 1, NORHP
IF( .NOT. LSHA(KHYD)) GO TO 101
RA1 = RA1 + RHPF(KHYD)
RA2 = RA2 + HSRX(KHYD)
RA3 = RA3 + RHPF(KHYD)*HSRX(KHYD)
RA4 = RA4 + RHPF(KHYD)**2
101 CONTINUE
AVRHPP = RA1/FNRHP
ASRX = RA2/FNRHP
RSLP = (RA3 - RA1*ASRX)/(RA4 - RA1**2/FNRHP)
IF(RSLP .LE. EPS) GO TO 106
RINT = ASRS - RSLP*AVRHPP
102 CSRX = RINT + RSLP*(0.5*CHCAP)
IF(CSRX .GE. 0.99) RETURN
IF(CSRX .LE. 0.8) CSRX = 0.8
GO TO 107
103 K1AH = 0
DO 104 KHYD = 1, NORHP
IF( .NOT. LSHA(KHYD)) GO TO 104
IF(K1AH .EQ. 0) K1AH = KHYD
IF(K1AH .GT. 0) K2AH = KHYD
104 CONTINUE
IF(NRHP .EQ. 1) GO TO 105
C FIT THE STRAIGHT LINE WHEN NRHP = 2
RSLP = (HSRX(K1AH) - HSRX(K2AH))/(RHPF(K1AH) - RHPF(K2AH))
IF(RSLP .LE. EPS) GO TO 106
RINT = HSRX(K1AH) - RSLP*RHPF(K1AH)
GO TO 102
105 CONTINUE
CSRX = HSRX(K1AH)
FSRX = CSRX
GO TO 115
106 CONTINUE
CSRX = SRX
FSRX = CSRX
WRITE(6,1)
1 FORMAT(//10X, 'REGRESSION LINE HAS NEGATIVE SLOPE')

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107 GO TO 115 SHM16740
CONTINUE SHM16750
BISRX = 0.2*(0.99 - CSRX) SHM16760
SISRX = 0.04*(0.99-CSRX) SHM16770
TFSRX = CSRX SHM16780
KISRX = 0 SHM16790
108 KISRX = KISRX + 1 SHM16800
FSRX = TFSRX SHM16810
WRITE(6,2) KISRX,CSRX,FSRX,CHCAP SHM16820
2 FORMAT(//15X,'TRIAL',I3,',',CSRX =',F8.5,',',FSRX =',F8.5,
1 ',',CHCAP =',F10.0) SHM16830
SQPKD = 0.0 SHM16840
ADRSP = 0.0 SHM16850
DO 109 KHYD = 1,NORHP SHM16860
IF(.NOT. LSHA(KHYD)) GO TO 109 SHM16870
KPCH = KPSH(KHYD) SHM16880
CALL TIMERT(SSR,SRR,CTRI,NCTRI,KHYD,KPCH) SHM16890
CALL STORRT(SRR,CSRX,FSRX,CHCAP,CONOP2,IBTPS,SHPF,KHYD,HBF(KHYD),
1 NHPT,KPCH,IBTPR) SHM16900
DRSP = SHPF - RHPF(KHYD) SHM16910
SQPKD = SQPKD + DRSP**2 SHM16920
ADRSP = ADRSP +DRSP SHM16930
109 CONTINUE SHM16940
WRITE(6,3) SQPKD SHM16950
3 FORMAT(//25X,'SQPKD =',F14.0) SHM16960
IF(KISRX .NE. 1) GO TO 110 SHM16970
TFSRX = CSRX + BISRX SHM16980
SSQPKD = SQPKD SHM16990
BFSRX = FSRX SHM17000
GO TO 108 SHM17010
110 IF(SQPKD .GT. SSQPKD) GO TO 113 SHM17020
IF(KISRX .EQ. 6 .AND. ADRSP .GT. 0.0) GO TO 111 SHM17030
SSQPKD = SQPKD SHM17040
BFSRX = FSRX SHM17050
IF(KISRX .GE. 11) GO TO 114 SHM17060
IF(KISRX .LE. 5) TFSRX = TFSRX +BISRX SHM17070
IF(KISRX .GE. 6) TFSRX =TFSRX -SISRX SHM17080
GO TO 108 SHM17090
111 KLCCA = KLCCA + 1 SHM17100
IF(KLCCA .GE. 5) GO TO 112 SHM17110
CHCAP = 0.8*CHCAP SHM17120
CSRX = RINT + RSLP*(0.5*CHCAP) SHM17130
GO TO 107 SHM17140
112 CSRX = 0.990 SHM17150
FSRX = 0.990 SHM17160
GO TO 115 SHM17170
113 IF(KISRX .GT. 6) GO TO 114 SHM17180
KISRX = 6 SHM17190
SSQPKD = SQPKD SHM17200
BFSRX = FSRX SHM17210
TFSRX = TFSRX -SISRX SHM17220
GO TO 108 SHM17230
114 FSRX = BFSRX SHM17240
WRITE(6,4) CSRX,FSRX,SSQPKD SHM17250
SHM17260
SHM17270

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4  FORMAT(//10X,'CSRX =',F7.4,10X,'FSRX =',F7.4,10X,'SQPKD =',F15.2) SHM17280
RETURN SHM17290
115  WRITE(6,5) CSRX,FSRX SHM17300
5  FORMAT(//10X,'CSRX =',F7.4,10X,'FSRX =',F7.4) SHM17310
RETURN SHM17320
END SHM17330
SUBROUTINE SET1RC(RSTF,KRS,NDRSC,BFRC) SHM17340
SETS BEST VALUE FOR ONE RECESSION CONSTANT SHM17350
DIMENSION RSTF(50,20) SHM17360
RA1 = 0.0 SHM17370
RA2 = 0.0 SHM17380
NDRSC1 = NDRSC -1 SHM17390
DO 100 KSD = 1,NDRSC1 SHM17400
RA1 = RA1 + RSTF(KSD,KRS)**2 SHM17410
100  RA2 = RA2 + RSTF(KSD,KRS)*RSTF(KSD+1,KRS) SHM17420
BFRC = RA2/RA1 SHM17430
WRITE(6,1) KRS,BFRC SHM17440
1  FORMAT(15X,'KRS =',I3,5X,'BFRC =',F8.4) SHM17450
RETURN SHM17460
END SHM17470
SUBROUTINE SET2RC(RSTF,KRS,NDRSC,IFRC,BFRC,LBFO) SHM17480
SETS BEST VALUES FOR TWO RECESSION CONSTANTS SHM17490
DIMENSION RSTF(50,20) SHM17500
LOGICAL LBFO SHM17510
REAL IFRC SHM17520
REAL*8 RA1,RA2,RA3,RA4,RA5,CRSTF(50),RA6,DBFRC,DIFRC,RA,RB,RD SHM17530
DO 100 KSD = 1,NDRSC SHM17540
100  CRSTF(KSD) = RSTF(KSD,KRS) SHM17550
NDRSC2 = NDRSC -2 SHM17560
RA1 = 0.0 SHM17570
RA2 = 0.0 SHM17580
RA3 = 0.0 SHM17590
DO 101 KSD = 1,NDRSC2 SHM17600
RA1 = RA1 + CRSTF(KSD)**2 SHM17610
RA2 = RA2 + CRSTF(KSD)*CRSTF(KSD+1) SHM17620
101  RA3 = RA3+CRSTF(KSD)*CRSTF(KSD+2) SHM17630
RA4 = RA1 +CRSTF(NDRSC-1)**2 -CRSTF(1)**2 SHM17640
RA5 = RA2 + CRSTF(NDRSC-1)*CRSTF(NDRSC) - CRSTF(1)*CRSTF(2) SHM17650
RA6 = RA4*RA1 -RA2**2 SHM17660
IF(RA6 .EQ. 0.0) GO TO 102 SHM17670
RA5 = RA5/RA6 SHM17680
RA3 = RA3/RA6 SHM17690
RA = RA1*RA5 - RA2*RA3 SHM17700
RB = RA4*RA3 - RA2*RA5 SHM17710
RD = RA**2 + 4.0*RB SHM17720
IF(RD .LT. 0.0) GO TO 102 SHM17730
DBFRC = (RA +RD **0.5)/2.0 SHM17740
DIFRC = RA -DBFRC SHM17750
BFRC = DBFRC SHM17760
IFRC = DIFRC SHM17770
WRITE(6,1) KRS,BFRC,IFRC SHM17780
1  FORMAT(15X,'KRS =',I3,5X,'BFRC =',F8.4,5X,'IFRC =',F8.4) SHM17790
GO TO 103 SHM17800
102  LBFO = .TRUE. SHM17810

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      WRITE(6,2) KRS SHM17820
2      FORMAT(/15X,'IMAGINARY VALUES ENCOUNTERED IN SET2RC, SEQUENCE =', SHM17830
1      13) SHM17840
103    RETURN SHM17850
      END SHM17860
      SUBROUTINE STORRT(SRR,CSRX,FSRX,CHCAP,CONOP2,IBTPS,SHPF,KHYD, SHM17870
1      CHBF,NHPT,KPCH,IBTPR) SHM17880
      PERFORMS CHANNEL STORAGE ROUTING SHM17890
      DIMENSION ASRR(5,21),SRR(5,170) SHM17900
      INTEGER CONOP2,PRD SHM17910
      REAL NHPT SHM17920
      WRITE(6,1) CHBF SHM17930
1      FORMAT(/25X,'BASE FLOW =',F7.1,' CFS') SHM17940
      TFCFS = CHBF SHM17950
      INHPT = NHPT SHM17960
      MHTP = 1 SHM17970
      IF(CONOP2 .EQ. 0) MHTP = 4 SHM17980
      INHPT = MHTP*INHPT SHM17990
      SHPF = 0.0 SHM18000
      RHF0 = 0.9*SRR(KHYD,1) SHM18010
      KAFH = 0 SHM18020
      DO 102 KHPT = 1,KPCH SHM18030
      PRD = 0 SHM18040
100    PRD = PRD + 1 SHM18050
      TRHF = SRR(KHYD,KHPT) SHM18060
      IF(TFCFS .LE. 0.5*CHCAP) SRX = CSRX SHM18070
      IF((TFCFS .GT. 0.5*CHCAP) .AND. (TFCFS .LT. 2.0*CHCAP)) SRX = SHM18080
1      CSRX + (FSRX -CSRX)*((TFCFS -0.5*CHCAP)/(1.5*CHCAP))**3 SHM18090
      IF(TFCFS .GE. 2.0*CHCAP) SRX = FSRX SHM18100
      RHF1 = TRHF- SRX*(TRHF -RHF0) SHM18110
      RHF0 = RHF1 SHM18120
      TFCFS = RHF1 + CHBF SHM18130
      IF(TFCFS .LT. SHPF) GO TO 101 SHM18140
      SHPF = TFCFS SHM18150
      IBTPS = KHPT SHM18160
101    IF(PRD .LE. 3 .AND. CONOP2 .EQ. 1) GO TO 100 SHM18170
      KAHF = KHPT - IBTPR*MHTP + 5*INHPT SHM18180
      IF(KAHF .LT. 0) GO TO 102 SHM18190
      IF(MOD(KAHF,INHPT) .NE. 0) GO TO 102 SHM18200
      KAFH = KAFH + 1 SHM18210
      ASRR(KHYD,KAFH) = TFCFS SHM18220
102    CONTINUE SHM18230
      IF(KAFH .EQ. 21) GO TO 104 SHM18240
      KAFH = KAFH + 1 SHM18250
      DO 103 KIA = KAFH,21 SHM18260
103    ASRR(KHYD,KIA) = 0.0 SHM18270
104    WRITE(6,2) KHYD,NHPT,(ASRR(KHYD,KWD), KWD = 1,21) SHM18280
2      FORMAT(/25X,'SYNTHESIZED HYDROGRAPH',I3,' INTERVAL =',F5.2, SHM18290
1      'HOURS'/3(22X,7F10.1/)) SHM18300
      WRITE(6,3) SHPF SHM18310
3      FORMAT(25X,'FLOOD PEAK =',F10.1,' CFS') SHM18320
      RETURN SHM18330
      END SHM18340
      SUBROUTINE STRHRS(RHPD,RPHH,IDYB,IDX,IRHR,IRRE,NHPT,MXTRH,DPY, SHM18350

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1 NRHP,IBTPR) SHM18360
 SITS BEGINNING AND END TIMES OF RUNOFF ENTERING RECORED HYDROGRAPHS SHM18370
 DIMENSION RHP(5),RPH(5),IDYB(5),IDYE(5),IHRB(5),IHRE(5) SHM18380
 INTEGER DAY,DPY,RHPD,RPH SHM18390
 REAL NHPT SHM18400
 ESTIMATE HOURS EACH WAY FROM PEAK SHM18410
 INHPT = NHPT SHM18420
 IBTPR = 5*INHPT + MXTRH SHM18430
 IPTE = 15*INHPT SHM18440
 DETERMINE TIME OF BEGINNING AND ENDING FOR EACH STORM SHM18450
 DO 106 KRHP = 1, NRHP SHM18460
 KHBCK = IBTPR - RPH(KRHP) SHM18470
 IF(KHBCK .LT. 0) GO TO 101 SHM18480
 KDBCK = KHBCK/24 + 1 SHM18490
 IHRB(KRHP) = 24*KDBCK-KHBCK SHM18500
 DAY = RHPD(KRHP) SHM18510
 100 DAY = DAY - 1 SHM18520
 IF(DAY.EQ. 59 .AND. DPY .EQ. 366) DAY = 366 SHM18530
 IF(DAY .EQ. 365) DAY = 59 SHM18540
 IF(DAY .EQ. 0) DAY = 365 SHM18550
 KDBCK = KDBCK - 1 SHM18560
 IF(KDBCK .GT. 0) GO TO 100 SHM18570
 IDYB(KRHP) = DAY SHM18580
 GO TO 102 SHM18590
 101 IDYB(KRHP) = RHPD(KRHP) SHM18600
 IHRB(KRHP) = RPH(KRHP) - IBTPR SHM18610
 102 KHFOR = IPTE + RPH(KRHP) SHM18620
 IF(KHFOR .LE. 24) GO TO 105 SHM18630
 KDFOR = KHFOR/24 SHM18640
 IHRE(KRHP) = KHFOR - 24*KDFOR SHM18650
 IF(IHRE(KRHP) .NE. 0) GO TO 103 SHM18660
 KDFOR = KDFOR - 1 SHM18670
 103 DAY = RHPD(KRHP) SHM18680
 104 CALL DAYNXT(DAY,DPY) SHM18690
 KDFOR = KDFOR - 1 SHM18700
 IF(KDFOR .GT. 0) GO TO 104 SHM18710
 IDYE(KRHP) = DAY SHM18720
 GO TO 106 SHM18730
 105 IDYE(KRHP) = RHPD(KRHP) SHM18740
 IHRE(KRHP) = RPH(KRHP) + IPTE SHM18750
 106 CONTINUE SHM18760
 C ELIMINATE HYDROGRAPH OVERLAPPING SHM18770
 NRHP1 = NRHP - 1 SHM18780
 IF(NRHP1 .EQ. 0) GO TO 109 SHM18790
 DO 108 KRHP = 1, NRHP1 SHM18800
 IF((IDYE(KRHP) .GT. IDYB(KRHP+1) .AND. (.NOT. ((IDYE(KRHP) .GE. 1 274 .AND. IDYB(KRHP+1) .LE. 273) .OR. IDYE(KRHP) .EQ. 366))) .OR. 2 (IDYE(KRHP) .EQ. IDYB(KRHP+1)).AND.IHRE(KRHP) .GT. IHRB(KRHP+1) 3)) GO TO 107 SHM18810
 GO TO 108 SHM18820
 107 IDYE(KRHP) = IDYB(KRHP+1) SHM18830
 IHRE(KRHP) = IHRB(KRHP+1) SHM18840
 108 CONTINUE SHM18850
 109 IF(IDYB(1) .LE. 273 .AND. RHPD(1) .GE. 274 .AND. RHPD(1) .NE. 366) SHM18860
 SHM18870
 SHM18880
 SHM18890

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1 GO TO 110 SHM18900
  GO TO 111 SHM18910
110 IDYB(1) = 274 SHM18920
  IHRB(1) = 1 SHM18930
111 IF(IDYE(NRHP) .GE. 274 .AND. RHPD(NRHP) .LE. 273 .AND. IDYE(NRHP)
1 .NE. 366) GO TO 112 SHM18940
  GO TO 113 SHM18950
112 IDYE(NRHP) = 273 SHM18960
  IHRE(NRHP) = 24 SHM18970
113 CONTINUE SHM18980
  DO 114 KRHP = 1,NRHP SHM18990
  WRITE(6,1) KRHP, IDYB(KRHP), IHRB(KRHP), IDYE(KRHP), IHRE(KRHP)
1 FORMAT(5X, 'RUNOFF CONTRIBUTING TO RECORDED HYDROGRAPH', I2/10X,
1 'BEGINS ON DAY', I4, ' AT HOUR', I3/10X, 'AND ENDS ON DAY', I4,
2 ' AT HOUR', I3) SHM19000
114 CONTINUE SHM19010
  RETURN SHM19020
  END SHM19030
  SUBROUTINE TIMERT(SSR,SRR,CTRI,NCTRI,KRHP,KPCH) SHM19040
  PERFORMS CHANNEL TIME ROUTING SHM19050
  DIMENSION SSR(5,170),SRR(5,170),CTRI(99) SHM19060
  DO 100 KHPT = 1,KPCH SHM19070
100 SRR(KRHP,KHPT) = 0.0 SHM19080
  KTRI = 1 SHM19090
101 CONTINUE SHM19100
  DO 102 KHPT = KTRI,KPCH SHM19110
  NRTRI = KHPT - KTRI + 1 SHM19120
102 SRR(KRHP,KHPT) = CTRI(KTRI)*SSR(KRHP,NRTRI) + SRR(KRHP,KHPT) SHM19130
  KTRI = KTRI + 1 SHM19140
  IF(KTRI .LE. NCTRI) GO TO 101 SHM19150
  RETURN SHM19160
  END SHM19170
  SUBROUTINE DAYNXT(DAY,DPY) SHM19180
C DETERMINES NUMBER OF NEXT DAY OF THE YEAR SHM19190
  INTEGER DAY,DPY SHM19200
  DAY = DAY+1 SHM19210
  IF(DAY .EQ. 366) DAY = 1 SHM19220
  IF(DAY .EQ. 60 .AND. DPY .EQ. 366) DAY = 366 SHM19230
  IF(DAY .EQ. 367) DAY=60 SHM19240
  RETURN SHM19250
  END SHM19260
  SUBROUTINE EVPDAY(DPET,EMAET) SHM19270
C DETERMINES DATED PAN EVAPORATION TOTALS SHM19280
  DIMENSION DPET(366) SHM19290
  RETURN SHM19300
  END SHM19310
  SUBROUTINE DAYOUT(VDCY,MEDWY,DPY) SHM19320
C PRINTS TABLE OF DAILY VALUES SHM19330
  DIMENSION MEDWY(12),VDCY(366),VDMC(12) SHM19340
  INTEGER DATE,DAY,DPY SHM19350
100 WRITE(6,1) SHM19360
  1 FORMAT(7X,3HDAY,7X,3HJUN,5X,3HJUL,5X,3HAUG,5X,4HSEPT,5X,3HOCT,5X,
1 3HNOV,5X,3HDEC,5X,3HJAN,5X,3HFEB,5X,3HMAR,5X,3HAPR,5X,3HMAY) SHM19370
  1 MEDWY(3)=0 SHM19380

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DO 104 DATE = 1,28,1 SHM19440
IF(MOD(DATE,5) .NE. 1) GO TO 102 SHM19450
DO 101 KMO = 1,12 SHM19460
DAY = MEDWY(KMO) + DATE SHM19470
101 VDMD(KMO) = VDCY(DAY) SHM19480
WRITE(6,2) DATE,VDMD(12),(VDMD(KWD),KWD=1,11) SHM19490
2 FORMAT(1H0,3X,I6,3X,12F8.1) SHM19500
GO TO 104 SHM19510
102 DO 103 KMO=1,12 SHM19520
DAY=MEDWY(KMO) + DATE SHM19530
103 VDMD(KMO) = VDCY(DAY) SHM19540
WRITE(6,3) DATE,VDMD(12),(VDMD(KWD),KWD=1,11) SHM19550
3 FORMAT(1X,3X, I6,3X,12F8.1) SHM19560
104 CONTINUE SHM19570
IF(DPY .NE. 366) GO TO 106 SHM19580
DATE = 29 SHM19590
VDCY(60) = VDCY(366) SHM19600
DO 105 KMO = 1,12 SHM19610
DAY = MEDWY(KMO) + DATE SHM19620
105 VDMD(KMO) = VDCY(DAY) SHM19630
WRITE(6,3) DATE,VDMD(12),(VDMD(KWD), KWD=1,11) SHM19640
GO TO 107 SHM19650
106 CONTINUE SHM19660
WRITE(6,4) VDCY(302),VDCY(333),VDCY(363),VDCY(29),VDCY(88), SHM19670
1VDCY(119),VDCY(149),VDCY(180),VDCY(210),VDCY(241),VDCY(272) SHM19680
4 FORMAT(1X,7X, 2H29,3X,4F8.1,8X,7F8.1) SHM19690
107 CONTINUE SHM19700
108 WRITE(6,5) VDCY(303),VDCY(334),VDCY(364),VDCY(30),VDCY(89), SHM19710
1VDCY(120),VDCY(150),VDCY(181),VDCY(211),VDCY(242),VDCY(273) SHM19720
5 FORMAT(1X,7X, 2H30,3X,4F8.1,8X,7F8.1) SHM19730
WRITE(6,6) VDCY(304),VDCY(365),VDCY(31),VDCY(90),VDCY(151), SHM19740
1VDCY(212),VDCY(243) SHM19750
6 FORMAT(1H/,7X,2H31,3X,F8.1,8X,2F8.1,8X,F8.1,8X,F8.1,8X,2F8.1) SHM19760
MEDWY(3) = 365 SHM19770
RETURN SHM19780
END SHM19790
SUBROUTINE PREPRD(RGPM,DRHP,DAY,HOUR,DPY,PRD,PEP,PRH) SHM19800
C DIVIDES HOURLY PRECIPITATION TOTALS AMONG PERIODS FOR SMALL BASINS SHM19810
DIMENSION PE4P(4),DRHP(366,24) SHM19820
INTEGER DAY,DPY,HOUR,PRD SHM19830
PEP=0.0 SHM19840
IF(PRH .EQ.0.0) RETURN SHM19850
IF(PRD .EQ. 1) GO TO 100 SHM19860
PEP = PE4P(PRD) SHM19870
RETURN SHM19880
100 LHOUR = HOUR-1 SHM19890
LDAY = DAY SHM19900
IF(LHOUR .GE. 1 ) GO TO 101 SHM19910
LHOUR = 24 SHM19920
LDAY=DAY-1 SHM19930
IF(LDAY .EQ. 0) LDAY=365 SHM19940
IF(LDAY .EQ. 365) LDAY=59 SHM19950
IF(LDAY .EQ. 59 .AND. DPY .EQ. 366) LDAY=366 SHM19960
101 PRLH = RGPM*DRHP(LDAY,LHOUR) SHM19970

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NHOUR = HOUR+1 SHM19980
NDAY = DAY SHM19990
IF(NHOUR .LE. 24) GO TO 102 SHM20000
NHOUR=1 SHM20010
CALL DAYNXT(NDAY,DPY) SHM20020
102 PRNH=RGPM*DRHP(NDAY,NHOUR) SHM20030
IF(PRH .GT. PRLH .AND. PRH .GT. PRNH) GO TO 103 SHM20040
GO TO 104 SHM20050
103 PE4P(1) = 0.10 SHM20060
PE4P(2) = 0.28 SHM20070
PE4P(3) = 0.46 SHM20080
PE4P(4) = 0.16 SHM20090
GO TO 108 SHM20100
104 IF(PRH .LT. PRLH .AND. PRH .LT. PRNH) GO TO 105 SHM20110
GO TO 106 SHM20120
105 PE4P(1) = 0.28 SHM20130
PE4P(2) = 0.10 SHM20140
PE4P(3) = 0.16 SHM20150
PE4P(4) = 0.46 SHM20160
GO TO 108 SHM20170
106 IF(PRNH .GE. PRLH) GO TO 107 SHM20180
PE4P(1) = 0.46 SHM20190
PE4P(2) = 0.16 SHM20200
PE4P(3) = 0.28 SHM20210
PE4P(4) = 0.10 SHM20220
GO TO 108 SHM20230
107 PE4P(1) = 0.10 SHM20240
PE4P(2) = 0.28 SHM20250
PE4P(3) = 0.16 SHM20260
PE4P(4) = 0.46 SHM20270
108 DO 109 KPRD = 1,4 SHM20280
109 PE4P(KPRD) = PE4P(KPRD)*PRH SHM20290
PEP=PE4P(1) SHM20300
RETURN SHM20310
END SHM20320